



U.S. Department  
of Transportation  
**Federal Highway  
Administration**

August 5, 2022

1200 New Jersey Ave., SE  
Washington, D.C. 20590

In Reply Refer To:  
HSST-1/B-366

Thomas Cho  
Triborough Bridge and Tunnel Authority (TBTA)  
Engineering and Construction, Room 215,  
Robert Mosses Building, Randall's Island,  
New York City, NY 10035  
USA

Dear Mr. Cho:

We received your correspondence of October 26, 2021 requesting issuance of a reimbursement eligibility letter under the Federal-aid highway program for the roadside safety system, device, design, product, or hardware (collectively "device") described below. This letter is assigned Federal Highway Administration (FHWA) control number B-366.

### **ELIGIBILITY LETTERS**

The FHWA issues Federal-aid reimbursement eligibility letters for new roadside safety devices that are crash tested in accordance with the industry standard of the American Association of State Highway and Transportation Officials (AASHTO) Manual for Assessing Safety Hardware (MASH).

FHWA, the Department of Transportation, and the United States (government) do not regulate roadside safety devices, crash test facilities, or the manufacturing industry. Issuance of eligibility letters is discretionary and provided only as a service to the states. FHWA may, at its discretion, decline to issue, revise, or rescind an eligibility letter. Eligibility letters are only issued by the FHWA headquarters Office of Safety.

Eligibility letters are issued only as notice to the states that a device is eligible for reimbursement under the Federal-aid highway program. They do not establish approval or certification for any other purpose. Issuance of an eligibility letter is not a prerequisite or requirement for state transportation agencies seeking to use Federal-aid funds for roadside safety devices. State agencies may use a device for which an eligibility letter has not been issued and seek Federal-aid reimbursement.

### **FEDERAL-AID REIMBURSEMENT**

The request for issuance of this letter certified the device was crash tested in accordance with the industry standard of AASHTO's MASH. This eligibility letter is based on that certification and the material offered in support of its issuance. The device described below is eligible for reimbursement under the Federal-aid highway program.

Name of system: RK-19 Bridge Rail  
Type of system: Bridge Rail  
Test Level: Test Level 5  
Testing conducted by: Texas A&M Transportation Institute  
Date of request: October 26, 2021

Information about the device, including material such as the eligibility request, crash test reports, drawings, or images are included in one or more attachment(s) to this letter.

In accordance with FHWA's Memo "Federal-aid Reimbursement Eligibility Process for Safety Hardware Devices" dated November 12, 2015, FHWA will make note of any reported damage to a test vehicle's fuel tank, oil pan, or other feature that might serve as a surrogate of the fuel tank. AASHTO's MASH states "Although not a specific factor in assessing test results, integrity of a test vehicle's fuel tank is a potential concern. It is preferable that the fuel tank remains intact and not be punctured. Damage or rupture of the fuel tank, oil pan, or other feature that might serve as a surrogate of the fuel tank should be reported". The test report included in this submittal states right fuel tank was damaged in Test 5-12.

Eligibility letter B-366 is inapplicable to devices, optional equipment, alternate materials, or other features that were not crash tested in accordance with AASHTO's MASH.

This letter is issued only for the subject device as crash tested under AASHTO's MASH. Later modification(s) of the device are not eligible for Federal-aid reimbursement under this letter. Notice of later modification(s) should be given to transportation agencies, facility owners, and operators (collectively "agencies").

Agencies should be provided appropriate information about the device's design, installation, maintenance, materials, and mechanical properties.

Issuance of this letter is discretionary, and it may be revised or rescinded at FHWA's discretion. This letter is not a determination of compliance with the Manual on Uniform Traffic Control Devices for Streets and Highways (MUTCD) or ownership of any intellectual property rights.

This eligibility letter is not a determination by the government that a crash involving the subject device will result in any particular outcome. It is limited to only the device's eligibility for Federal-aid reimbursement.

### **INTELLECTUAL PROPERTY**

Issuance of this eligibility letter does not convey property rights of any sort nor any exclusive privilege. This letter is not authorization or consent by the government for the use, manufacture, or sale of any patented or proprietary system, device, design, product, or hardware for which the requester is not the patent owner. Eligibility letters are not an expression of any view, position, or determination by the government as to the validity, scope, or ownership of any intellectual property rights to a specific device. These letters do not grant, impute, suggest, or otherwise

establish any ownership, distribution, or licensing rights to the requester. The government expresses no opinion about the intellectual property rights relating to any device for which this or any other eligibility letter is issued.

**PUBLIC DISCLOSURE**

To prevent any misunderstanding, and as discussed above, this eligibility letter is assigned FHWA control number B-366. It should only be reproduced in full with its attachment(s). This letter and the material offered by the requester supporting its issuance is public information. All eligibility letters and supporting material are subject to public disclosure under the Freedom of Information Act (FOIA). Eligibility letters are available to the public at [https://safety.fhwa.dot.gov/roadway\\_dept/countermeasures/reduce\\_crash\\_severity/](https://safety.fhwa.dot.gov/roadway_dept/countermeasures/reduce_crash_severity/).

If you have any questions please contact Aimee Zhang at [Aimee.Zhang@dot.gov](mailto:Aimee.Zhang@dot.gov).

Sincerely,

A handwritten signature in black ink that reads "Michael S. Griffith". The signature is written in a cursive style with a large, stylized "S" in the middle.

Michael S. Griffith  
Director, Office of Safety Technologies  
Office of Safety

Enclosures

## Request for Federal Aid Reimbursement Eligibility of Highway Safety Hardware

<b>Submitter</b>	Date of Request:	October 26, 2021	<input checked="" type="radio"/> New <input type="radio"/> Resubmission
	Name:	Thomas Cho	
	Company:	Triborough Bridge and Tunnel Authority (TBTA)	
	Address:	Engineering and Construction, Room 215, Robert Mosses Building, Randall's Island, New York, NY 10035	
	Country:	USA	
	To:	Michael S. Griffith, Director FHWA, Office of Safety Technologies	

I request the following devices be considered eligible for reimbursement under the Federal-aid highway program.

**Device & Testing Criterion** - Enter from right to left starting with Test Level

!-!-!

System Type	Submission Type	Device Name / Variant	Testing Criterion	Test Level
'B': Rigid/Semi-Rigid Barriers (Roadside, Median, Bridge Railings)	<input checked="" type="radio"/> Physical Crash Testing <input type="radio"/> Engineering Analysis	RK-19 Bridge Rail	AASHTO MASH	TL5

By submitting this request for review and evaluation by the Federal Highway Administration, I certify that the product(s) was (were) tested in conformity with the AASHTO Manual for Assessing Safety Hardware and that the evaluation results meet the appropriate evaluation criteria in the MASH.

**Individual or Organization responsible for the product:**

Contact Name:	Thomas Cho	Same as Submitter <input checked="" type="checkbox"/>
Company Name:	Triborough Bridge and Tunnel Authority (TBTA)	Same as Submitter <input checked="" type="checkbox"/>
Address:	Engineering and Construction, Room 215, Robert Mosses Building, Randall's Island, New York, NY 10035	Same as Submitter <input checked="" type="checkbox"/>
Country:	USA	Same as Submitter <input checked="" type="checkbox"/>
Enter below all disclosures of financial interests as required by the FHWA 'Federal-Aid Reimbursement Eligibility Process for Safety Hardware Devices' document.		
Texas A&M Transportation Institute (TTI) was contracted by Ammann & Whitney / WSP on behalf of TBTA to perform full-scale crash testing of the RK-19 Bridge Rail. There are no shared financial interests in the RK-19 Bridge Rail by TTI, or between Ammann & Whitney / WSP and TTI, other than costs involved in the actual crash tests and reports for this submission to FHWA.		
**614911-01-1, -2, -3**		

### PRODUCT DESCRIPTION

<input checked="" type="radio"/> New Hardware or Significant Modification	<input type="radio"/> Modification to Existing Hardware
The installation consisted of a steel bridge rail system mounted to a 1ft-10 inches thick orthotropic steel bridge deck, with a 3/8-inch-thick overlay. The bridge deck was 9 ft wide, and 50 ft-9 inches long, starting 1 ft-8½ inches upstream of post 3 and ending 1 ft-8½ inches downstream of post 11. Steel W8x28 posts, which supported 1 rectangular and 3 square steel rails, were evenly spaced at 5 ft-11 inches center-to-center for a total length of 130 ft-2 inches. The top rail (HSS 6x6x½ inch) was mounted to the posts at 44-1/8 inches from the roadway to the top of the rail, the preceding two rails (HSS 6x6x5/16 inch) were mounted below with 5 inches between each rail, and the bottom rail (HSS 5x3x¾ inch) was mounted 6 inches below the rail above.	
<h3>CRASH TESTING</h3>	
By signature below, the Engineer affiliated with the testing laboratory, agrees in support of this submission that all of the critical and relevant crash tests for this device listed above were conducted to meet the MASH test criteria. The Engineer has determined that no other crash tests are necessary to determine the device meets the MASH criteria.	
Engineer Name:	_____
Engineer Signature:	<div style="display: flex; align-items: center;"> <div style="font-size: 24pt; font-weight: bold; margin-right: 10px;">William Williams</div> <div style="font-size: 18pt; color: #c00000; margin-right: 10px;">}</div> <div>                     Digitally signed by William Williams                      Date: 2021.10.26 10:08:01 -05'00'                 </div> </div>
Address:	1254 Avenue A, Bldg 7091, Bryan, Texas 77807 USA Same as Submitter <input type="checkbox"/>
Country:	_____ Same as Submitter <input type="checkbox"/>

A brief description of each crash test and its result:


Required Test Number	Narrative Description	Evaluation Results
5-10 (1100C)	<p>Test 5-10 involves an 1100C vehicle impacting the test article at a target impact speed of 62 mi/h <math>\pm</math>2.5 mi/h and a target impact angle of 25° <math>\pm</math>1.5°. The target CIP was for the right corner of the front bumper to impact 3.6 ft upstream of the centerline of the rail splice between posts 6 and 7.</p> <p>The results of the test conducted on April 27, 2021, are found in TTI Test Report number 614911-01. The test vehicle was traveling at an impact speed of 62.4 mi/h as it made contact with the rail 3.5 ft upstream of the centerline of the rail splice between posts 6 and 7 at an impact angle of 25.0°. After loss of contact with the barrier, the vehicle came to rest 201 ft downstream of the impact point and 40 ft towards the traffic side.</p> <p>The bridge rail contained and redirected the 1100C vehicle. The vehicle did not penetrate, underride, or override the installation. The 1100C vehicle exited within the exit box criteria.</p> <p>Working width was 17.5 inches. Maximum dynamic deflection was 1.3 inches during the test. No permanent deformation was observed afterwards.</p> <p>No detached elements, fragments, or other debris were present to penetrate or show potential for penetrating the occupant compartment, or present hazard to others in the area.</p> <p>Maximum exterior crush to the vehicle was 10.0 inches in the front and side planes at the right front corner at and above bumper height. Maximum occupant compartment deformation was 3.0 inches in the right front firewall/toe pan area.</p> <p>The 1100C vehicle remained upright during and after the collision event. Maximum roll and pitch angles were 8° and 4°, respectively. Longitudinal OIV was 21.5 ft/s, and lateral OIV was 34.6 ft/s. Longitudinal occupant ridedown acceleration was 3.4 g, and lateral occupant ridedown acceleration 11.2 g. The occupant risk factors were within the MASH allowable limits.</p> <p>The RK-19 Bridge Rail performed acceptably for MASH test 5-10.</p>	PASS

Required Test Number	Narrative Description	Evaluation Results
5-11 (2270P)	<p>Test 5-11 involves a 2270P vehicle impacting the test article at a target impact speed of 62 mi/h <math>\pm</math>2.5 mi/h and a target impact angle of 25° <math>\pm</math>1.5°. The target CIP was for the right corner of the front bumper to impact at 4.3 ft upstream of the centerline of the rail splice between posts 6 and 7.</p> <p>The results of the test conducted on April 29, 2021 are found in TTI Test Report number 614911-01. The test vehicle was traveling at an impact speed of 64.0 mi/h as it made contact with the barrier 4.1 ft upstream of the centerline of the rail splice between posts 6 and 7 at an angle of 25.1°. After loss of contact with the barrier, the vehicle came to rest 241 ft downstream of the impact point and 31 ft towards the traffic side.</p> <p>The bridge rail contained and redirected the 2270P vehicle. The vehicle did not penetrate, underride, or override the installation. The 2270P vehicle exited within the exit box criteria.</p> <p>Working width was 17.5-inches. Maximum dynamic deflection during the test was 1.5 inches. No permanent deformation was observed afterwards.</p> <p>No detached elements, fragments, or other debris were present to penetrate or show potential for penetrating the occupant compartment, or present hazard to others in the area.</p> <p>Maximum exterior crush to the vehicle was 11.0 inches in the side plane at the right front corner above bumper height.</p> <p>Maximum occupant compartment deformation 1.0 inch in the right front fire wall/toe pan area and the right front kickpanel.</p> <p>The 2270P vehicle remained upright during and after the collision event. Maximum roll and pitch angles were 9° and 7°, respectively. Longitudinal OIV was 16.2 ft/s, and lateral OIV was 31.7 ft/s. Longitudinal occupant ridedown acceleration was 6.7 g, and lateral occupant ridedown acceleration 15.2 g. The occupant risk factors were within the MASH allowable limits.</p> <p>The RK-19 Bridge Rail performed acceptably for MASH test 5-11.</p>	PASS

5-12 (36000V)	<p>Test 5-12 involves a 36000V vehicle impacting the test article at a target impact speed of 50 mi/h <math>\pm</math>2.5 mi/h and a target impact angle of 15° <math>\pm</math>1.5°. The target CIP was for the right corner of the front bumper to impact at 1 ft downstream of the centerline of the rail splice between posts 6 and 7.</p> <p>The results of the test conducted on May 7, 2021 are found in TTI Test Report number 614911-01. The test vehicle was traveling at an impact speed of 51.8 mi/h as it made contact with the barrier 0.9 ft downstream of the centerline of the rail splice between posts 6 and 7 at an angle of 15.9°. After loss of contact with the barrier, the vehicle came to rest 233 ft downstream of the impact point and 91 ft towards the traffic side. The bridge rail contained and redirected the 36000V vehicle. The vehicle did not penetrate, underide, or override the installation. The 36000V vehicle exited within the exit box criteria.</p> <p>Working width was 46.5 inches. Maximum dynamic deflection during the test was 2.8 inches. The maximum permanent deformation was 1.0 inch</p> <p>No detached elements, fragments, or other debris were present to penetrate or show potential for penetrating the occupant compartment, or present hazard to others in the area</p> <p>Maximum exterior crush to the vehicle was 10.0 inches at the right front corner at bumper height. Maximum occupant compartment deformation was 4.0 inches in the right front floor pan.</p> <p>The 36000V vehicle remained upright during and after the collision event. .</p> <p>Maximum roll and pitch angles were 19° and 50°, respectively. Longitudinal OIV was 1.2 ft/s, and lateral OIV was 18.6 ft/s.</p> <p>Longitudinal occupant ridedown acceleration was 15.4 g, and lateral occupant ridedown acceleration 38.2 g.</p> <p>The RK-19 Bridge Rail performed acceptably for MASH test 5-12.</p>	PASS
5-20 (1100C)	This bridge rail is not a transition system.	Non-Relevant Test, not conducted
5-21 (2270P)	This bridge rail is not a transition system.	Non-Relevant Test, not conducted
5-22 (36000V)	This bridge rail is not a transition system.	Non-Relevant Test, not conducted



Full Scale Crash Testing was done in compliance with MASH by the following accredited crash test laboratory (cite the laboratory's accreditation status as noted in the crash test reports.):

Laboratory Name:	Texas AM Transportation Institute		
Laboratory Signature:	Digitally signed by Darrell L. Kuhn 'Date: 2021.10.26 09:46:42 -05'00		
Address:	1254 Avenue A, Bldg 7091, Bryan, Texas 77807 USA	Same as Submitter	<input type="checkbox"/>
Country:	USA	Same as Submitter	<input type="checkbox"/>
Accreditation Certificate Number and Dates of current Accreditation period :	ISO 17025-2017 Laboratory A2LA Certificate Number: 2821.01 Valid To: April 30, 2023		

Submitter Signature\*: Thomas Cho

Digitally signed by Thomas Cho  
DN: cn=Thomas Cho, o=MTA Construction & Development, ou=Engineering, email=thomas.cho@mtaccd.org, c=US  
Date: 2022.01.20 16:01:28 -05'00'

Submit Form

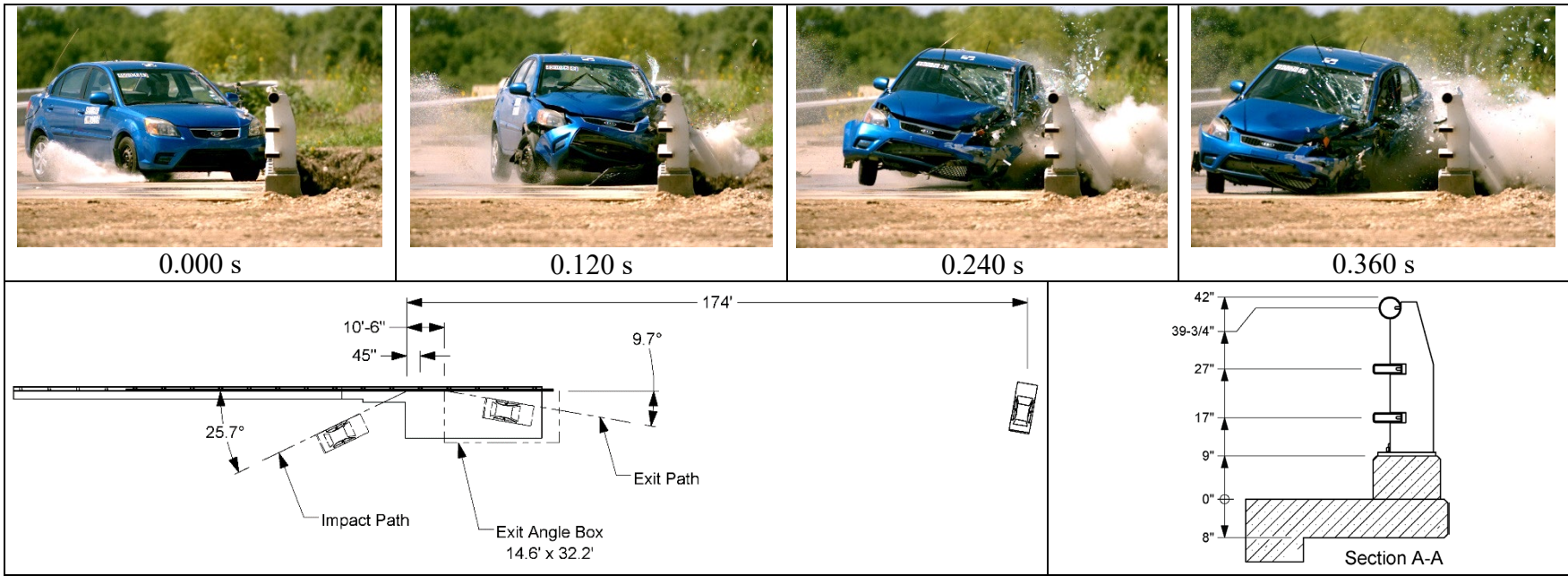
## ATTACHMENTS

Attach to this form:

- 1) Additional disclosures of related financial interest as indicated above.
- 2) A copy of the full test report, video, and a Test Data Summary Sheet for each test conducted in support of this request.
- 3) A drawing or drawings of the device(s) that conform to the Task Force-13 Drawing Specifications [[Hardware Guide Drawing Standards](#)]. For proprietary products, a single isometric line drawing is usually acceptable to illustrate the product, with detailed specifications, intended use, and contact information provided on the reverse. Additional drawings (not in TF-13 format) showing details that are relevant to understanding the dimensions and performance of the device should also be submitted to facilitate our review.

FHWA Official Business Only:

Eligibility Letter		Key Words
Number	Date	



**General Information**

Test Agency..... Texas A&M Transportation Institute (TTI)  
 Test Standard Test No..... MASH Test 4-10  
 TTI Test No. .... 490026-4-1  
 Test Date ..... 2016-07-20

**Test Article**

Type ..... Bridge Rail  
 Name ..... TxDOT Type C2P Bridge Rail  
 Installation Length..... 112 ft Rail Post-to-Post  
 Material or Key Elements .... Three steel rails supported on fabricated steel posts mounted on concrete curb and deck

**Soil Type and Condition** ..... Concrete Bridge Deck, Dry

**Test Vehicle**

Type/Designation ..... 1100C  
 Make and Model ..... 2010 Kia Rio  
 Curb..... 2493 lb  
 Test Inertial ..... 2433 lb  
 Dummy ..... 165 lb  
 Gross Static ..... 2598 lb

**Impact Conditions**

Speed .....63.0 mi/h  
 Angle .....25.7 degrees  
 Location/Orientation .....45 inches upstream of post 11

**Impact Severity** .....60 kip-ft

**Exit Conditions**

Speed .....49.5 mi/h  
 Angle .....9.7 degrees

**Occupant Risk Values**

Longitudinal OIV .....26.2 ft/s  
 Lateral OIV.....33.1 ft/s  
 Longitudinal Ridedown .....2.8 g  
 Lateral Ridedown .....8.2 g  
 THIV .....46.0 km/h  
 PHD .....8.5 g  
 ASI .....2.81

**Max. 0.050-s Average**

Longitudinal .....-14.9 g  
 Lateral.....19.7 g  
 Vertical.....-3.8 g

**Post-Impact Trajectory**

Stopping Distance..... 174 ft downstream  
 2 ft twd field side

**Vehicle Stability**

Maximum Yaw Angle .....52 degrees  
 Maximum Pitch Angle .....5 degrees  
 Maximum Roll Angle .....6 degrees  
 Vehicle Snagging .....No  
 Vehicle Pocketing .....No

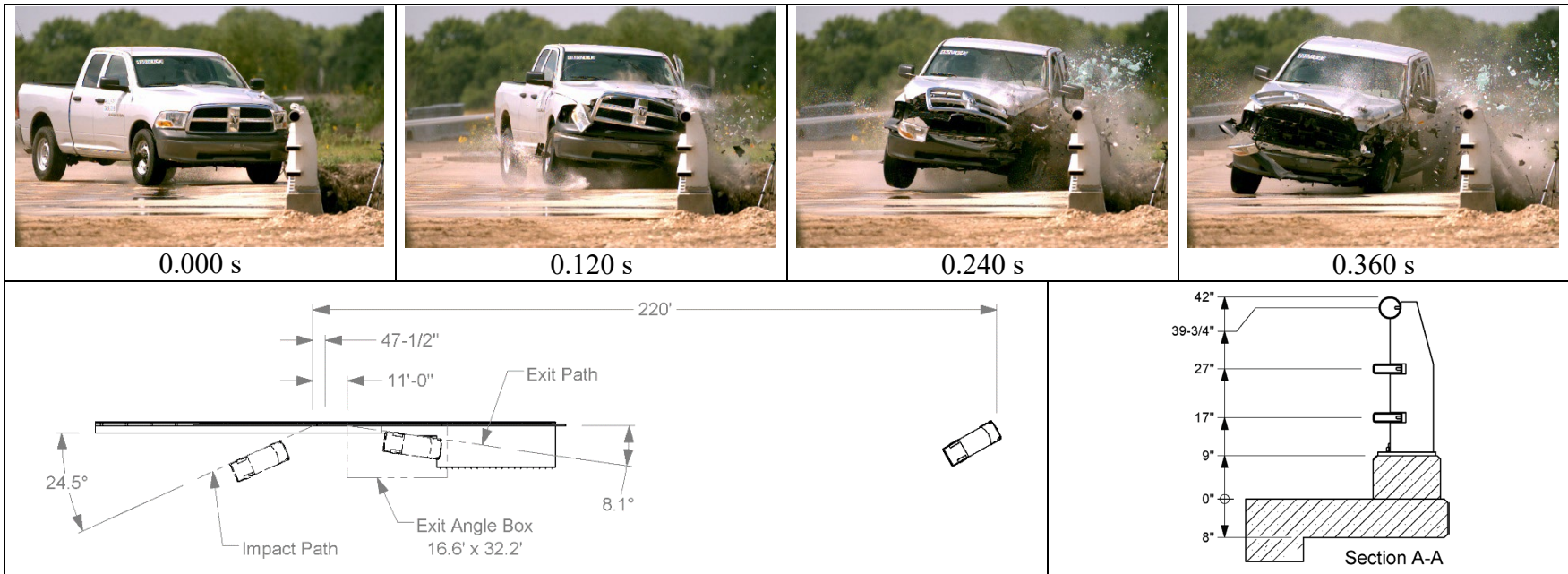
**Test Article Deflections**

Dynamic.....0.8 inch  
 Permanent .....None noted  
 Working Width.....14.0 inches

**Vehicle Damage**

VDS .....11LFQ5  
 CDC .....11FLEW4  
 Max. Exterior Deformation.....13.0 inches  
 OCDI.....LF0033000  
 Max. Occupant Compartment Deformation .....4.0 inches Driver's side toe pan

**Figure 5-1. Summary of Results for MASH Test 4-10 on TxDOT Type C2P Bridge Rail.**



**General Information**

Test Agency ..... Texas A&M Transportation Institute (TTI)  
 Test Standard Test No. .... MASH Test 4-11  
 TTI Test No. .... 490026-4-2  
 Test Date ..... 2016-07-13

**Test Article**

Type ..... Bridge Rail  
 Name ..... TxDOT Type C2P Bridge Rail  
 Installation Length ..... 112 ft Rail Post-to-Post  
 Material or Key Elements .... Three steel rails supported on fabricated steel posts mounted on concrete curb and deck

**Soil Type and Condition** ..... Concrete Bridge Deck, Dry

**Test Vehicle**

Type/Designation ..... 2270P  
 Make and Model ..... 2011 Dodge RAM 1500  
 Curb ..... 4911 lb  
 Test Inertial ..... 5048 lb  
 Dummy ..... 165 lb  
 Gross Static ..... 5213 lb

**Impact Conditions**

Speed ..... 62.9 mi/h  
 Angle ..... 24.5 degrees  
 Location/Orientation ..... 47½ inches upstream of post 6

**Impact Severity** ..... 115 kip-ft

**Exit Conditions**

Speed ..... 50.5 mi/h  
 Angle ..... 8.1 degrees

**Occupant Risk Values**

Longitudinal OIV ..... 18.4 ft/s  
 Lateral OIV ..... 29.5 ft/s  
 Longitudinal Ridedown ..... 3.0 g  
 Lateral Ridedown ..... 9.5 g  
 THIV ..... 38.7 km/h  
 PHD ..... 9.9 g  
 ASI ..... 2.03  
 Max. 0.050-s Average  
 Longitudinal ..... -9.9 g  
 Lateral ..... 15.3 g  
 Vertical ..... -2.5 g

**Post-Impact Trajectory**

Stopping Distance ..... 220 ft downstream  
 1 ft twd traffic lanes

**Vehicle Stability**

Maximum Yaw Angle ..... 37 degrees  
 Maximum Pitch Angle ..... 4 degrees  
 Maximum Roll Angle ..... 5 degrees  
 Vehicle Snagging ..... No  
 Vehicle Pocketing ..... No

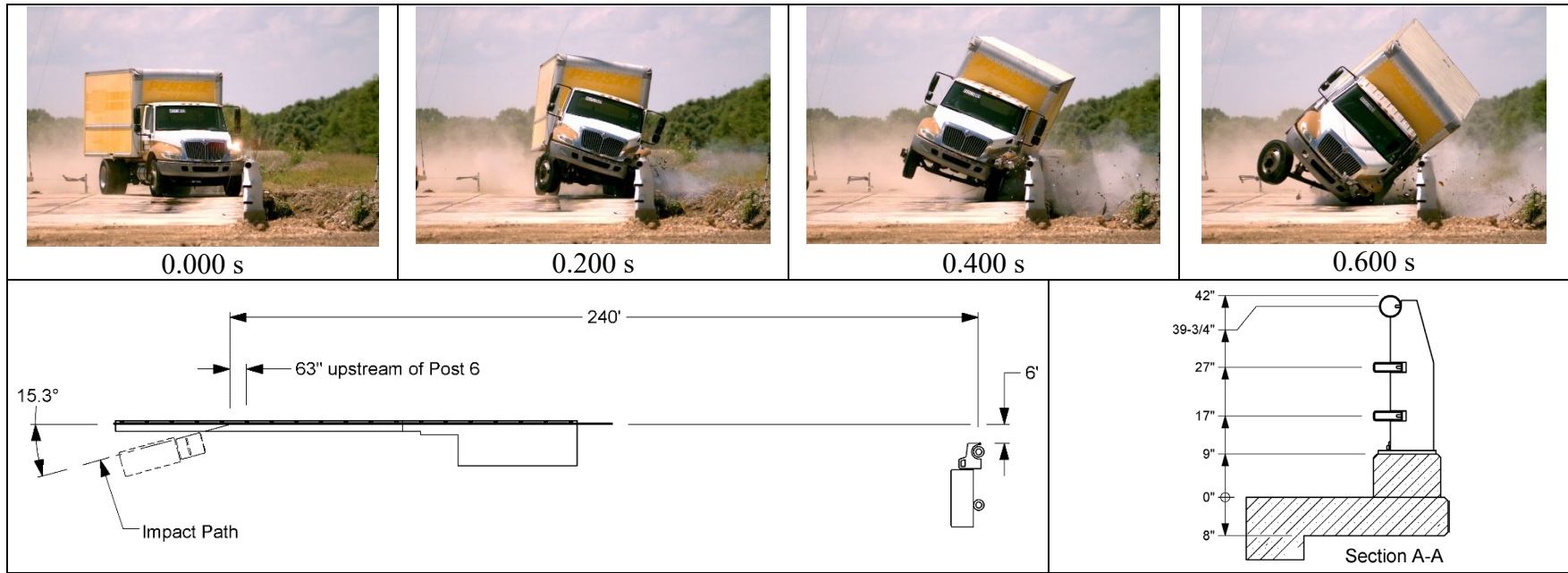
**Test Article Deflections**

Dynamic ..... 2.5 inches  
 Permanent ..... 1.4 inches  
 Working Width ..... 14.0 inches

**Vehicle Damage**

VDS ..... 11LFQ4  
 CDC ..... 11FLEW3  
 Max. Exterior Deformation ..... 13.0 inches  
 OCDI ..... LF0000000  
 Max. Occupant Compartment Deformation ..... 2.25 inches driver side dash.

**Figure 6-2. Summary of Results for MASH Test 4-11 on TxDOT Type C2P Bridge Rail.**



**General Information**

Test Agency..... Texas A&M Transportation Institute (TTI)  
 Test Standard Test No..... MASH Test 4-12  
 TTI Test No. .... 490026-4-3  
 Test Date ..... 2016-06-27

**Test Article**

Type ..... Bridge Rail  
 Name ..... TxDOT Type C2P Bridge Rail  
 Installation Length..... 144 ft Rail Post-to-Post  
 Material or Key Elements .... Three steel rails supported on fabricated steel posts mounted on concrete curb and deck

**Soil Type and Condition**

..... Concrete Bridge Deck, Dry

**Test Vehicle**

Type/Designation ..... 10000S  
 Make and Model ..... 2004 International 4200 single-unit box van truck  
 Curb..... 12,360 lb  
 Ballast..... 10,287  
 Test Inertial ..... 22,220 lb  
 Gross Static ..... 23,385 lb

**Impact Conditions**

Speed .....58.4 mi/h  
 Angle .....15.3 degrees  
 Location/Orientation .....63 inches upstream of post 6

**Impact Severity**

.....176 kip-ft

**Exit Conditions**

Speed .....54.1 mi/h  
 Angle .....Not obtainable

**Occupant Risk Values**

Longitudinal OIV .....6.2 ft/s  
 Lateral OIV.....15.1 ft/s  
 Longitudinal Ridedown .....3.6 g  
 Lateral Ridedown .....8.0 g  
 THIV .....17.8 km/h  
 PHD .....8.0 g  
 ASI .....0.61  
 Max. 0.050-s Average  
 Longitudinal .....-1.8 g  
 Lateral.....-5.4 g  
 Vertical.....-2.5 g

**Post-Impact Trajectory**

Stopping Distance.....240 ft dnwnstrm  
 6 ft twd traffic

**Vehicle Stability**

Maximum Yaw Angle .....29 degrees  
 Maximum Pitch Angle ..... 10 degrees  
 Maximum Roll Angle .....89 degrees  
 Vehicle Snagging .....No  
 Vehicle Pocketing .....No

**Test Article Deflections**

Dynamic..... 11.4 inches  
 Permanent ..... 7.25 inches  
 Working Width.....62.3 inches

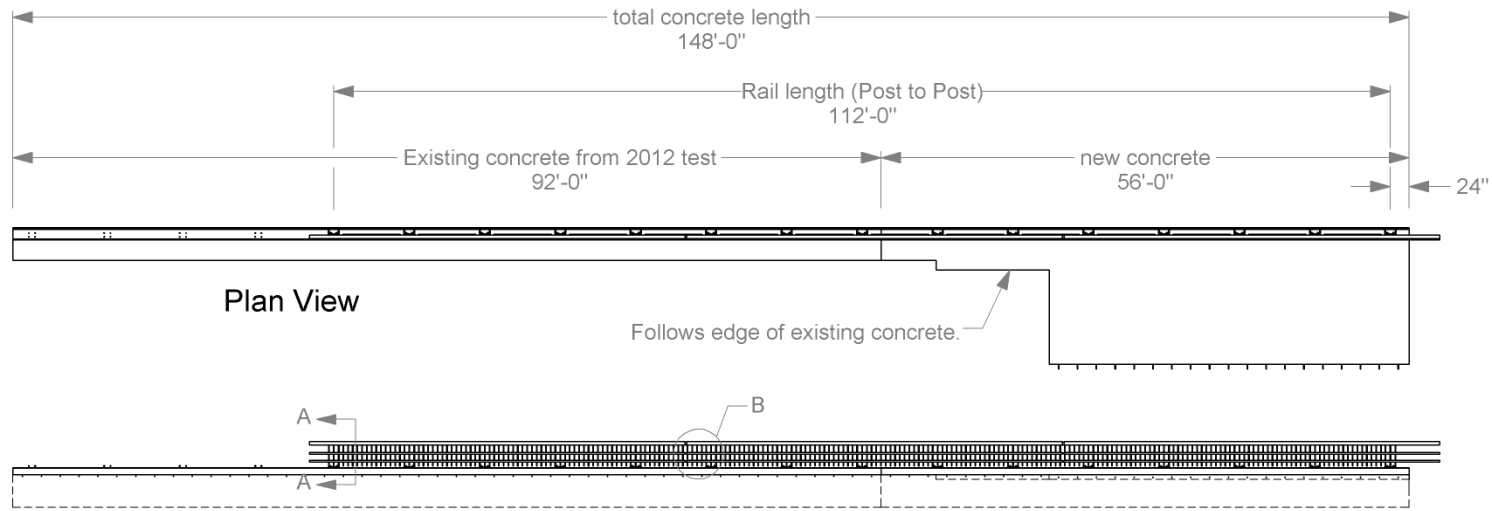
**Vehicle Damage**

VDS ..... NA  
 CDC..... 11FLEW5  
 Max. Exterior Deformation..... 14.0 inches  
 OCDI..... LF0000000  
 Max. Occupant Compartment Deformation .....None

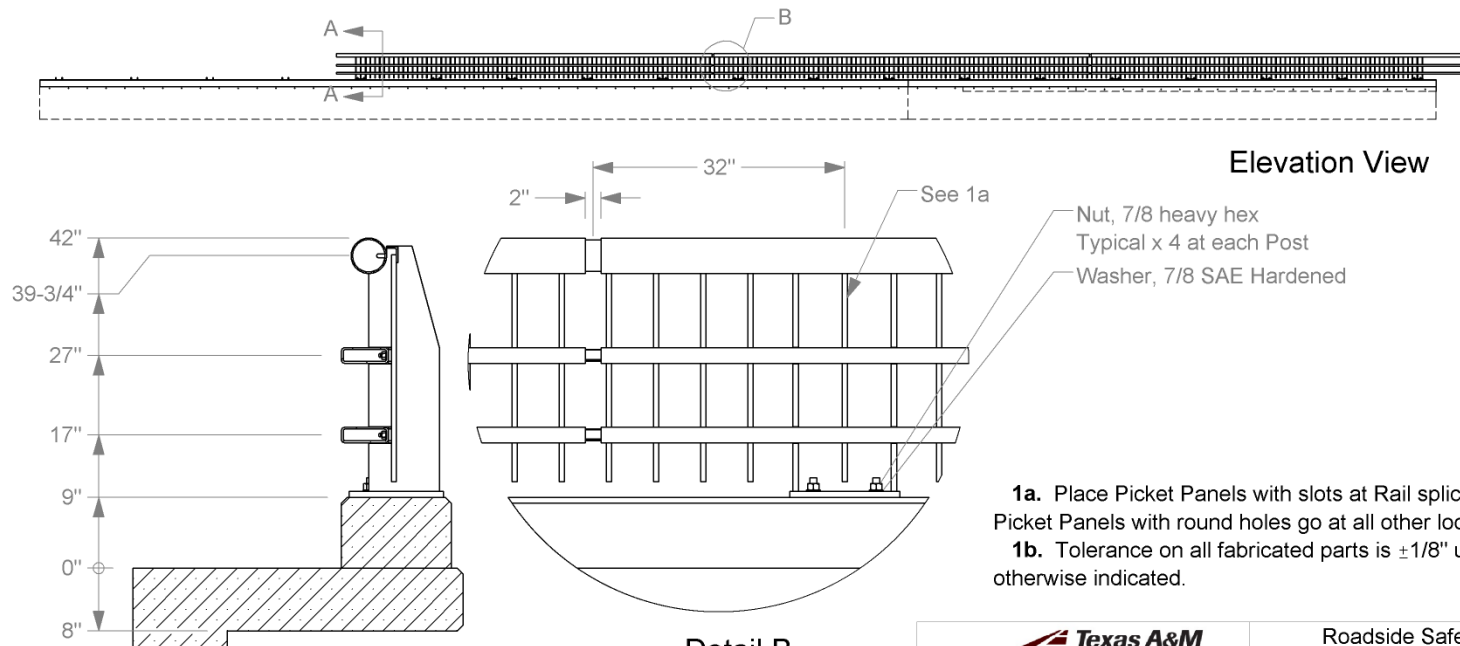
**Figure 7-3. Summary of Results for MASH Test 4-12 on TxDOT Type C2P Bridge Rail.**



# Test Installation



Plan View

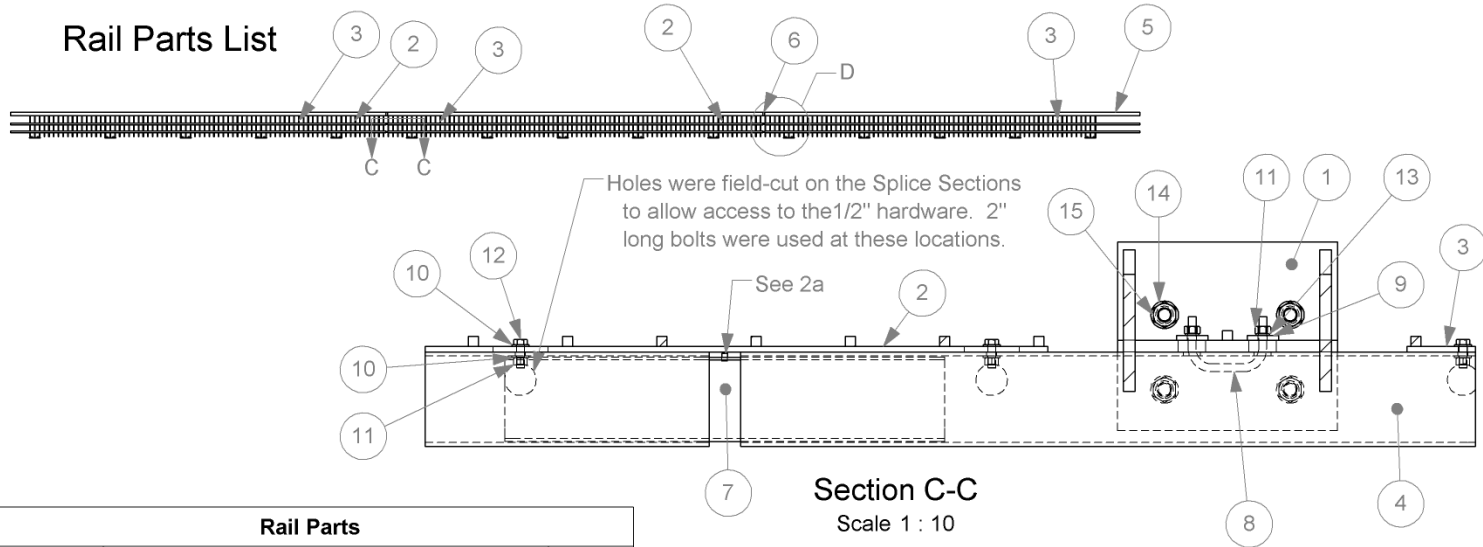


Elevation View

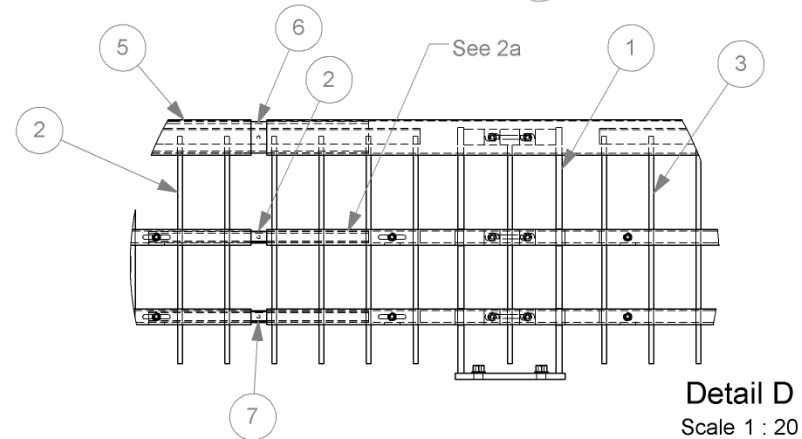
- 1a. Place Picket Panels with slots at Rail splice locations. Picket Panels with round holes go at all other locations.
- 1b. Tolerance on all fabricated parts is  $\pm 1/8"$  unless otherwise indicated.

		Roadside Safety and Physical Security Division - Proving Ground	
Project 490026-4 / 1-2 42" Picket Rail		2016-07-18	
Drawn By GES	Scale:1:200	Sheet 1 of 14	Test Installation

### Rail Parts List



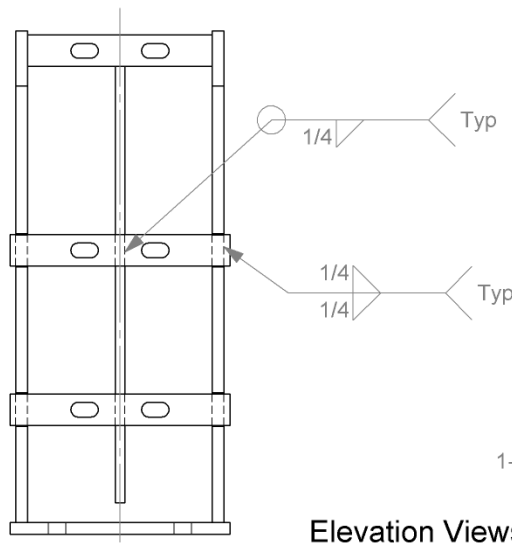
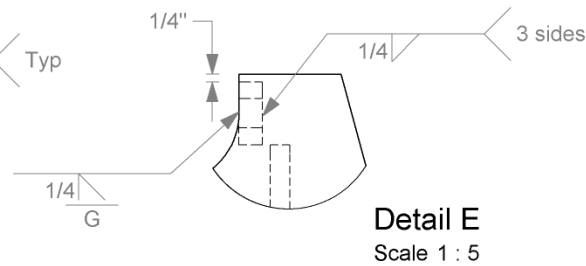
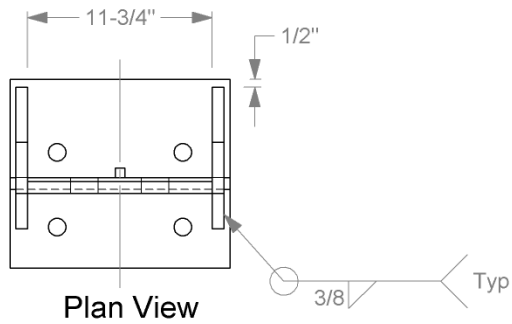
Rail Parts		
#	Part Name	Qty.
1	Post for 42" Picket Rail	15
2	Picket Rail Panel, with slots	2
3	Picket Rail Panel, with holes	12
4	Rectangular Rail	6
5	Round Rail	3
6	Splice Section for Round Rail	2
7	Splice Section for Rectangular Rail	4
8	U-bolt for Picket Rail	45
9	Plate Washer for U-bolt	90
10	Washer, 1/2 SAE Hardened	168
11	Nut, 1/2 heavy hex	174
12	Bolt, 1/2" x 1 1/2" hex	84
13	Washer, 1/2 Lock	90
14	Washer, 7/8 SAE Hardened	60
15	Nut, 7/8 heavy hex	60



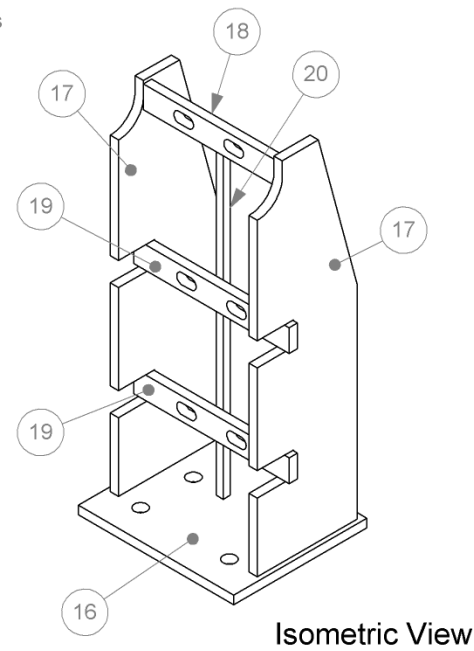
Roadside Safety and  
Physical Security Division -  
Proving Ground

Project 490026-4 / 1-2 42" Picket Rail 2016-07-18  
 Drawn By GES Scale:1:200 Sheet 2 of 14 Rail Parts List

# Post



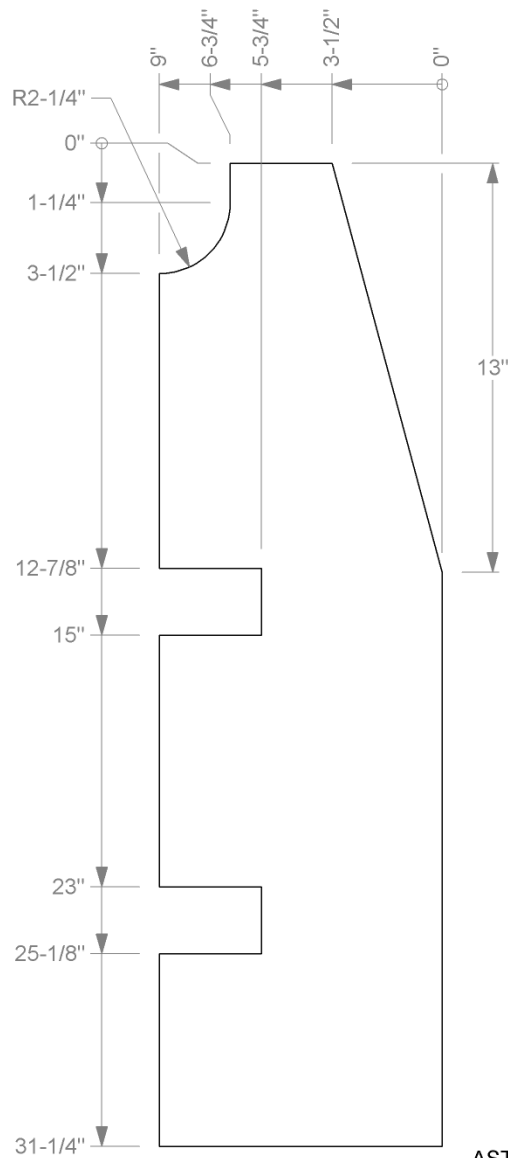
Post Parts					
#	Body Name	Description	Length	MATERIAL	Qty
16	Base Plate	Plate, 12" x 3/4"	14"	ASTM A572 Grade 50	1
17	Side Plate	Plate, 9" x 3/4"	31 1/4"	ASTM A572 Grade 50	2
18	Rail Plate, Top	Plate, 2" x 3/4"	11 3/4"	ASTM A36 Steel	1
19	Rail Plate, Bot and Mid	Plate, 2" x 3/4"	14"	ASTM A36 Steel	2
20	Picket	Plate, 5/8" x 5/8"	27 3/4"	ASTM A36 Steel	1



Roadside Safety and  
Physical Security Division -  
Proving Ground

Project 490026-4 / 1-2 42" Picket Rail 2016-07-18  
 Drawn By GES Scale:1:500 Sheet 3 of 14 Post

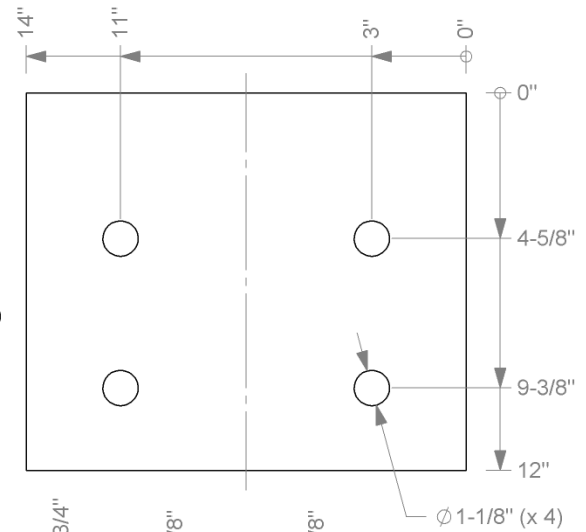




### Post Parts

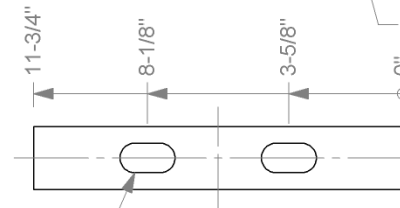
(See Table, previous sheet, for Picket details)

**Base Plate**  
Plate, 12" x 3/4"  
ASTM A572 Grade 50

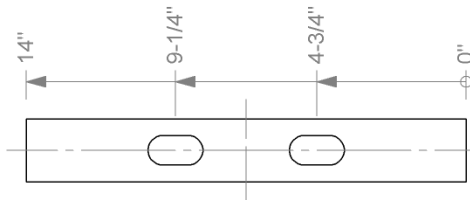


**Rail Plate, Top**  
Plate, 2" x 3/4"  
ASTM A36 Steel

Slots, 15/16" x 1-3/4"  
Typical both Rail Plates



**Rail Plate, Bot and Mid**  
Plate, 2" x 3/4"  
ASTM A36 Steel

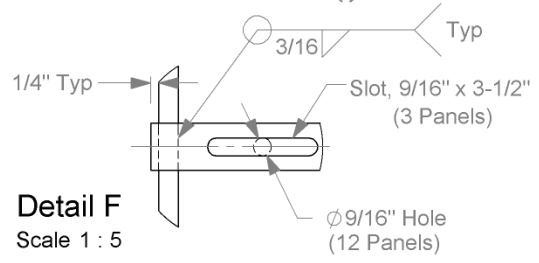
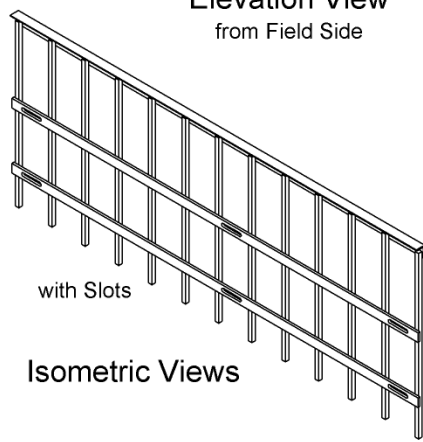
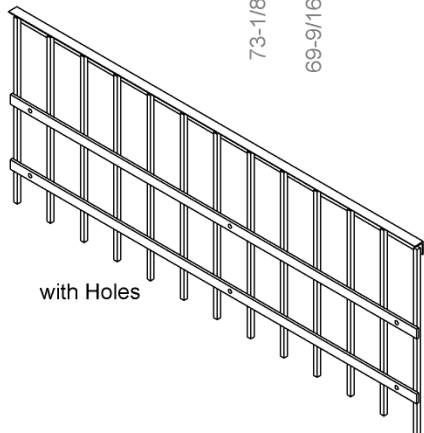
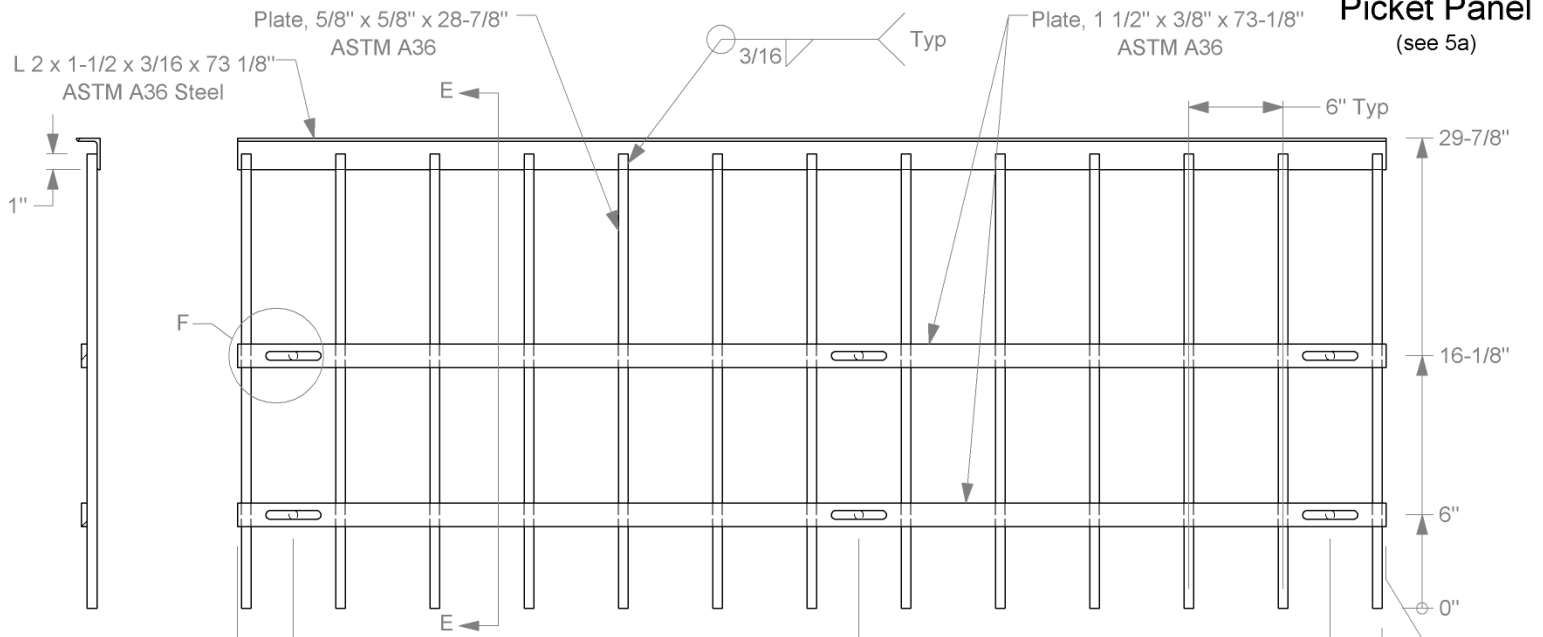


**Side Plate**  
Plate, 9" x 3/4"  
ASTM A572 Grade 50



Roadside Safety and  
Physical Security Division -  
Proving Ground

Project 490026-4 / 1-2	42" Picket Rail	2016-07-18
Drawn By GES	Scale 1:5	Sheet 4 of 14 Post Parts

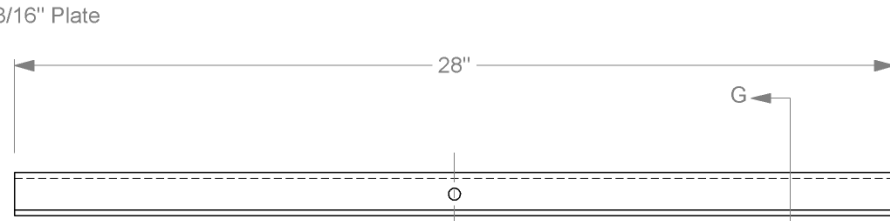
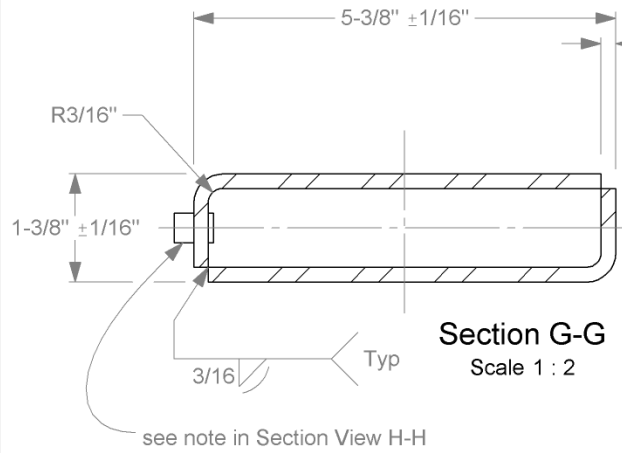


**5a.** Need 18 Panels total, 3 with Slots and 15 with holes at same location.

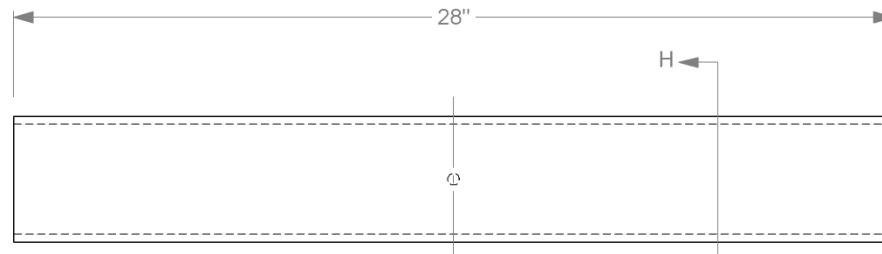
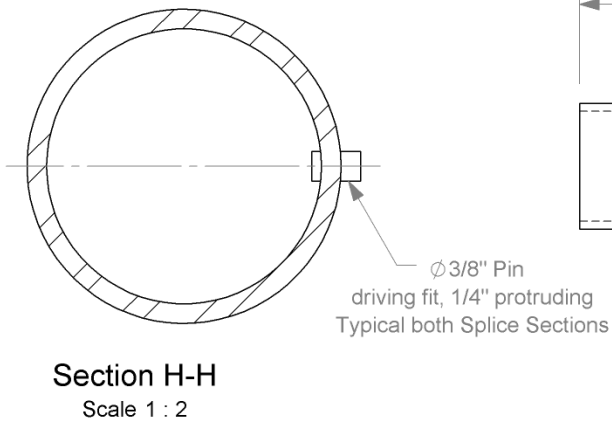
	Roadside Safety and Physical Security Division - Proving Ground	
	Project 490026-4 / 1-2 42" Picket Rail	2016-07-18
Drawn By GES	Scale 1:10	Sheet 5 of 14 Picket Panel

T:\11-ProjectFiles\490026-TxDOT\4 - 42 inch Picket Rail - Williams\4-2 Drafting\490026-4-2 Drawing

## Splice Sections



**Splice Section for Rectangular Rail**  
ASTM A36 Steel  
(see 6a)



**Splice Section for Round Rail**  
HSS Round 4" x 1/4" x 28"  
ASTM A500 Grade B

**6a.** Check Splice Sleeve for Rectangular Rail for loose fit in Rectangular Rail after fabrication is completed.

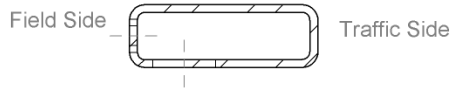
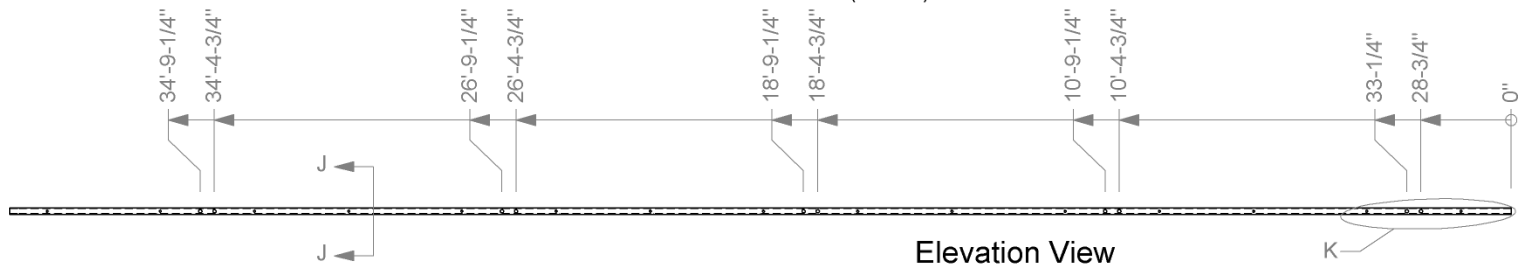
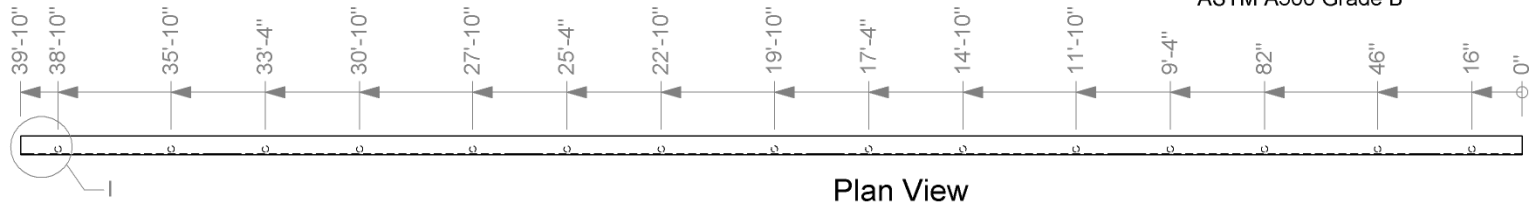


Roadside Safety and  
Physical Security Division -  
Proving Ground

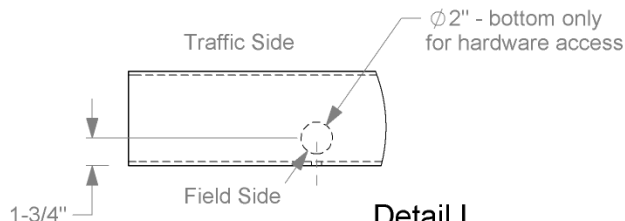
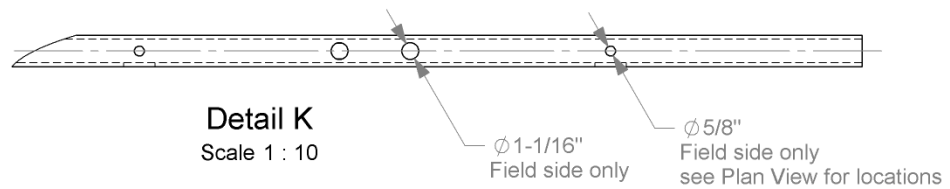
Project 490026-4 / 1-2 42" Picket Rail	2016-07-18
Drawn By GES	Scale 1:5 Sheet 6 of 14 Splice Sections

# Rectangular Rail

HSS 6" x 2" x 1/4"  
ASTM A500 Grade B

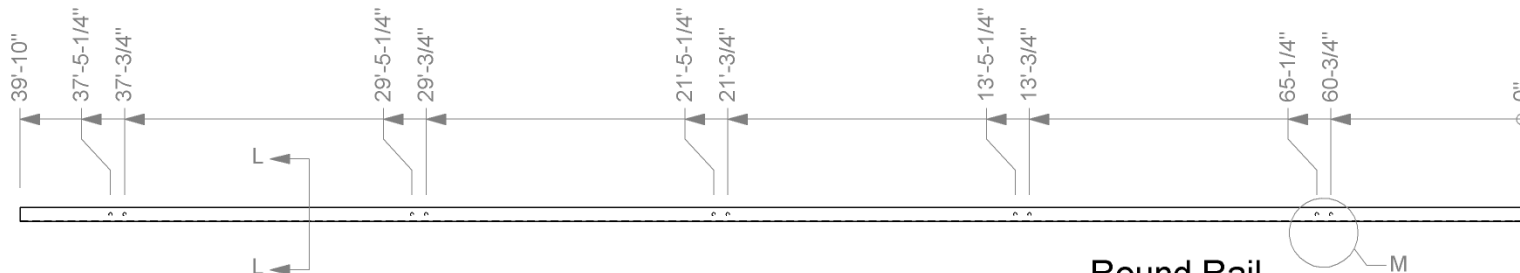


**Section J-J**  
Scale 1 : 5

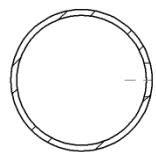


**7a.** Please note that the Plan View and Detail I are showing the Rail from the top. The holes are on bottom.

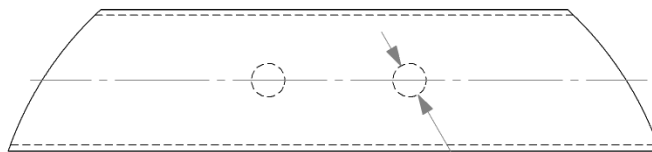
		Roadside Safety and Physical Security Division - Proving Ground	
Project 490026-4 / 1-2 42" Picket Rail		2016-07-18	
Drawn By GES	Scale 1:50	Sheet 7 of 14	Rectangular Rail



**Round Rail**  
 HSS Round 4 1/2" x 3/16"  
 ASTM A500 Grade B  
 Elevation View

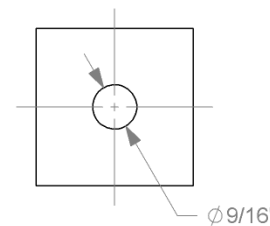


**Section L-L**  
 Scale 1 : 5

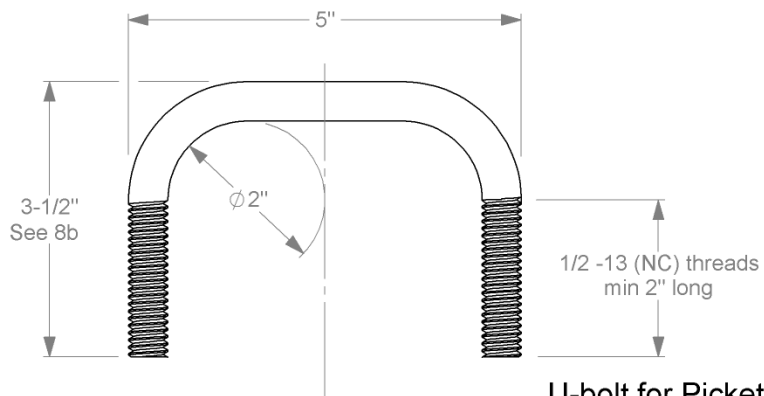


**Detail M**  
 Scale 1 : 5

Ø 1-1/16"  
 Field side only



**Plate Washer for U-bolt**  
 Plate, 2" x 5/16" x 2"  
 ASTM A36 Steel  
 Scale 1:2



**U-bolt for Picket Rail**  
 Ø 1/2" ASTM A36 Steel  
 Scale 1:2

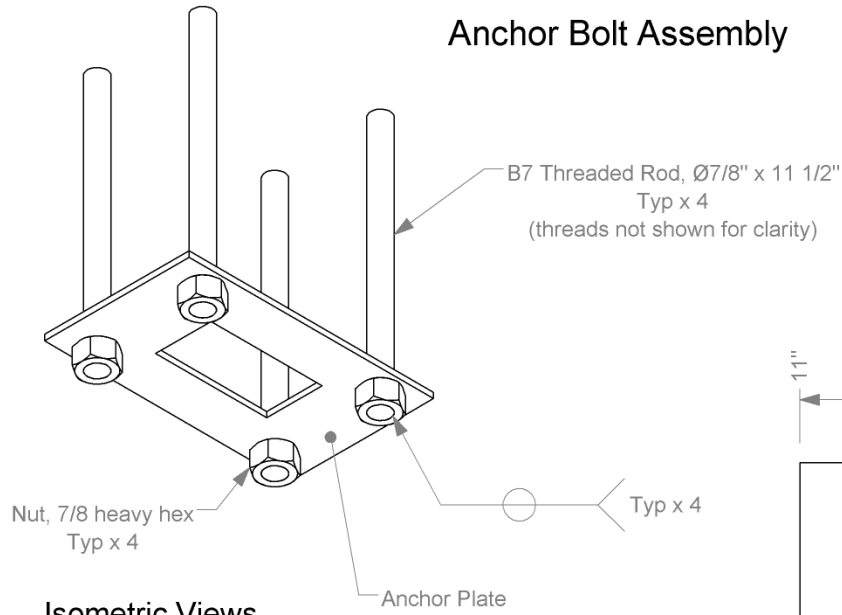
**8a.** Dimension is approximate. Rod is 10" long before bending.



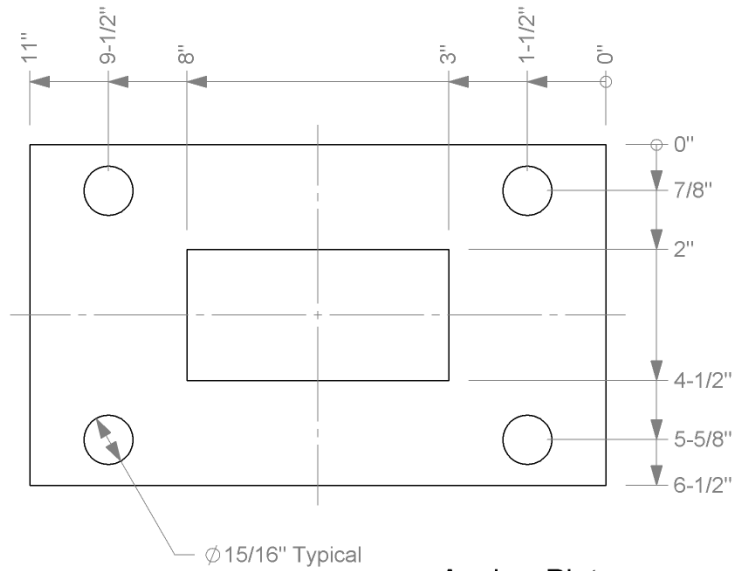
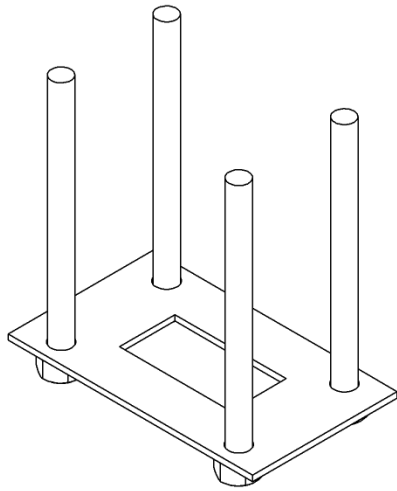
Roadside Safety and  
 Physical Security Division -  
 Proving Ground

Project 490026-4-3	42" Picket Rail	2016-07-20
Drawn By GES	Scale 1:50	Sheet 8 of 14 Round Rail, etc.

# Anchor Bolt Assembly



## Isometric Views

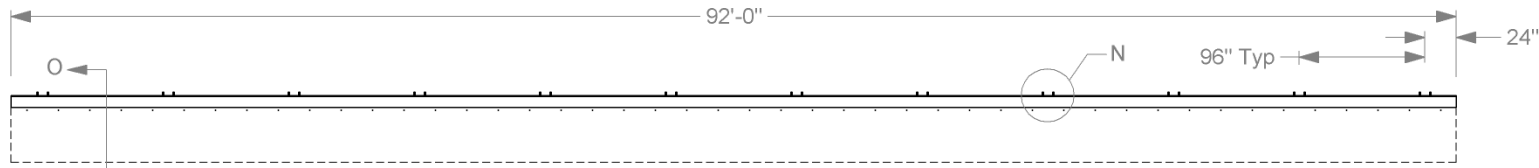


**Anchor Plate**  
Plate, 6 1/2" x 1/4" x 11"  
ASTM A36 Steel



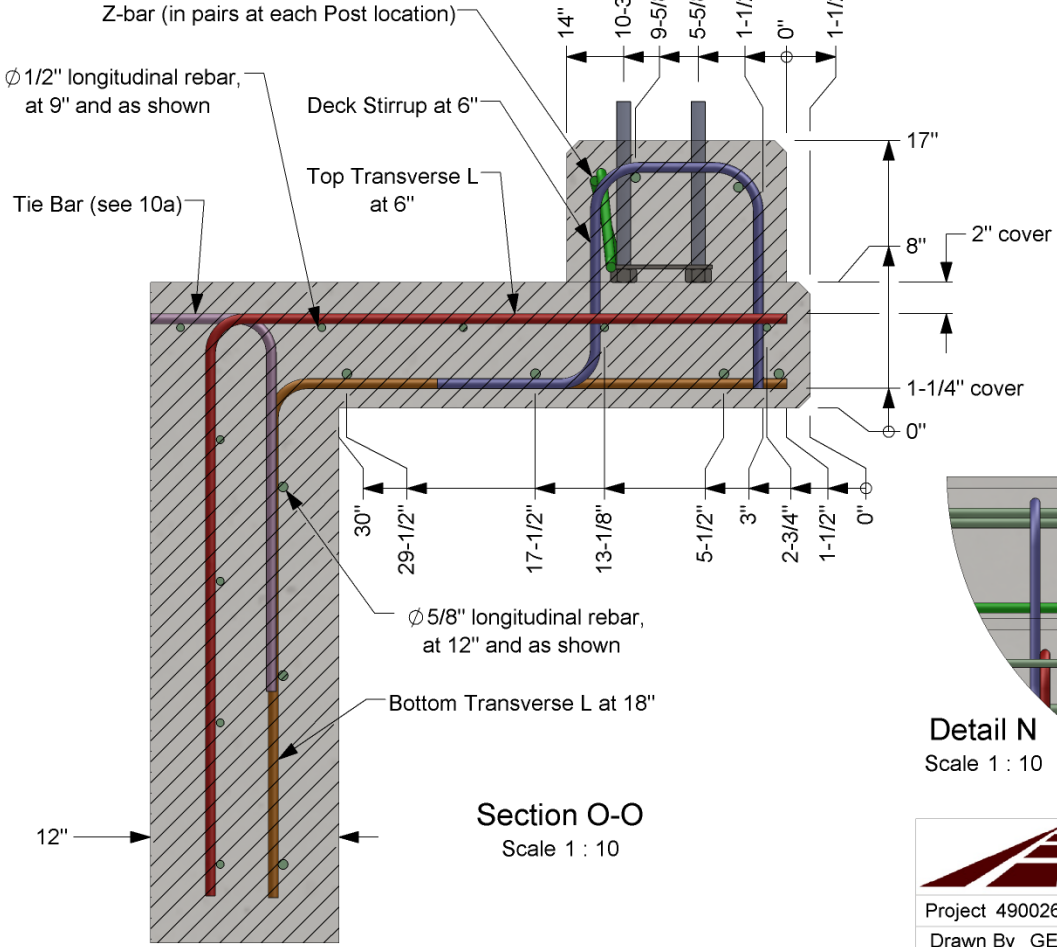
Roadside Safety and  
Physical Security Division -  
Proving Ground

Project 490026-4-3	42" Picket Rail	2016-07-20
Drawn By GES	Scale 1:3	Sheet 9 of 14
Anchor Bolt Assembly		



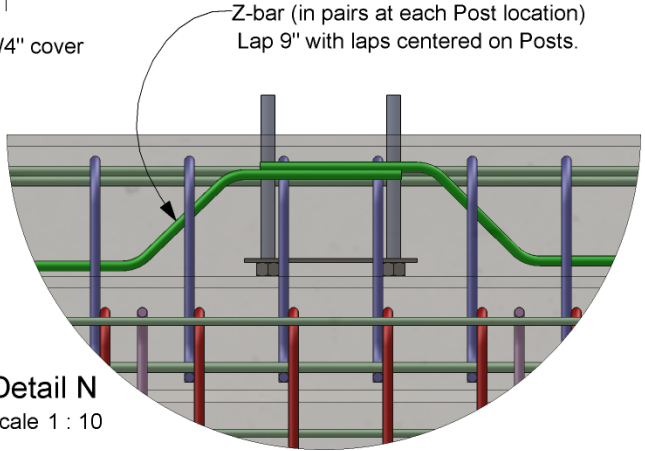
Elevation View

Previous Concrete



Section O-O  
Scale 1 : 10

**10a.** Tie Bars spaced at 24", and welded to existing rebar protruding from the runway (not shown here).  
**10b.** All Rebar is grade 60. Minimum lap distance is 17" for Ø1/2" bars and 21" for Ø5/8" bars.  
**10c.** Concrete is TxDOT Class S (4000 psi). Chamfer edges of Deck and Curb 3/4" as shown.

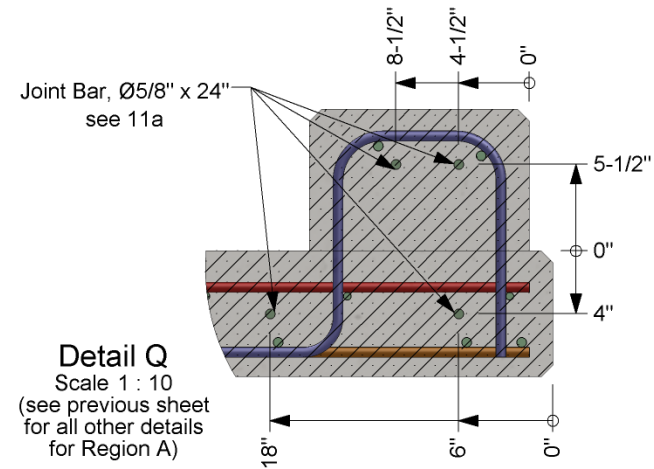
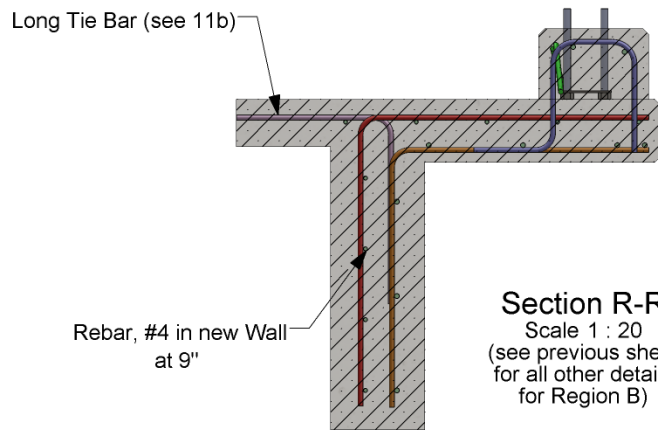
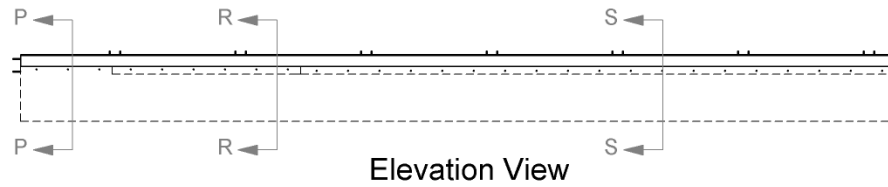
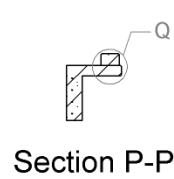
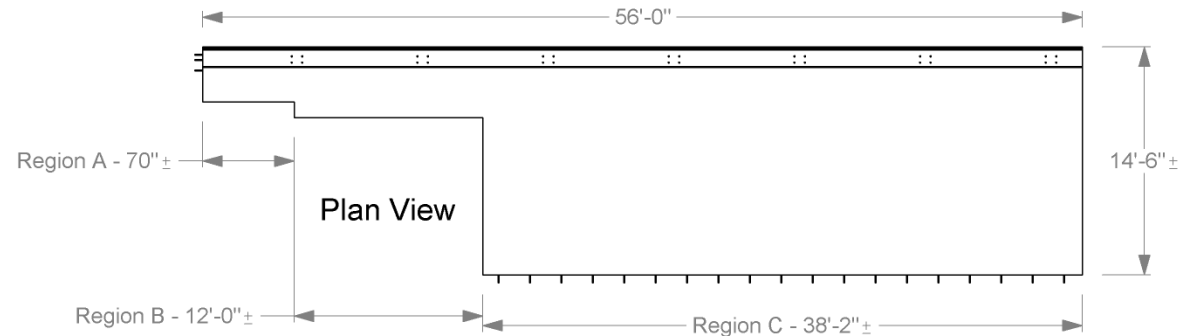


Detail N  
Scale 1 : 10

		Roadside Safety and Physical Security Division - Proving Ground	
Project 490026-4-3	42" Picket Rail	2016-07-20	
Drawn By GES	Scale: 1:120	Sheet 10 of 14	Previous Concrete

T:\11-ProjectFiles\490026-TxDOT\4 - 42 inch Picket Rail - Williams\4-3\Drafting\490026-4-3 Drawing

# New Concrete



**11a.** Drill minimum 6" into existing concrete and secure Joint Bars with Hilti RE-500 epoxy, according to manufacturer's instructions.

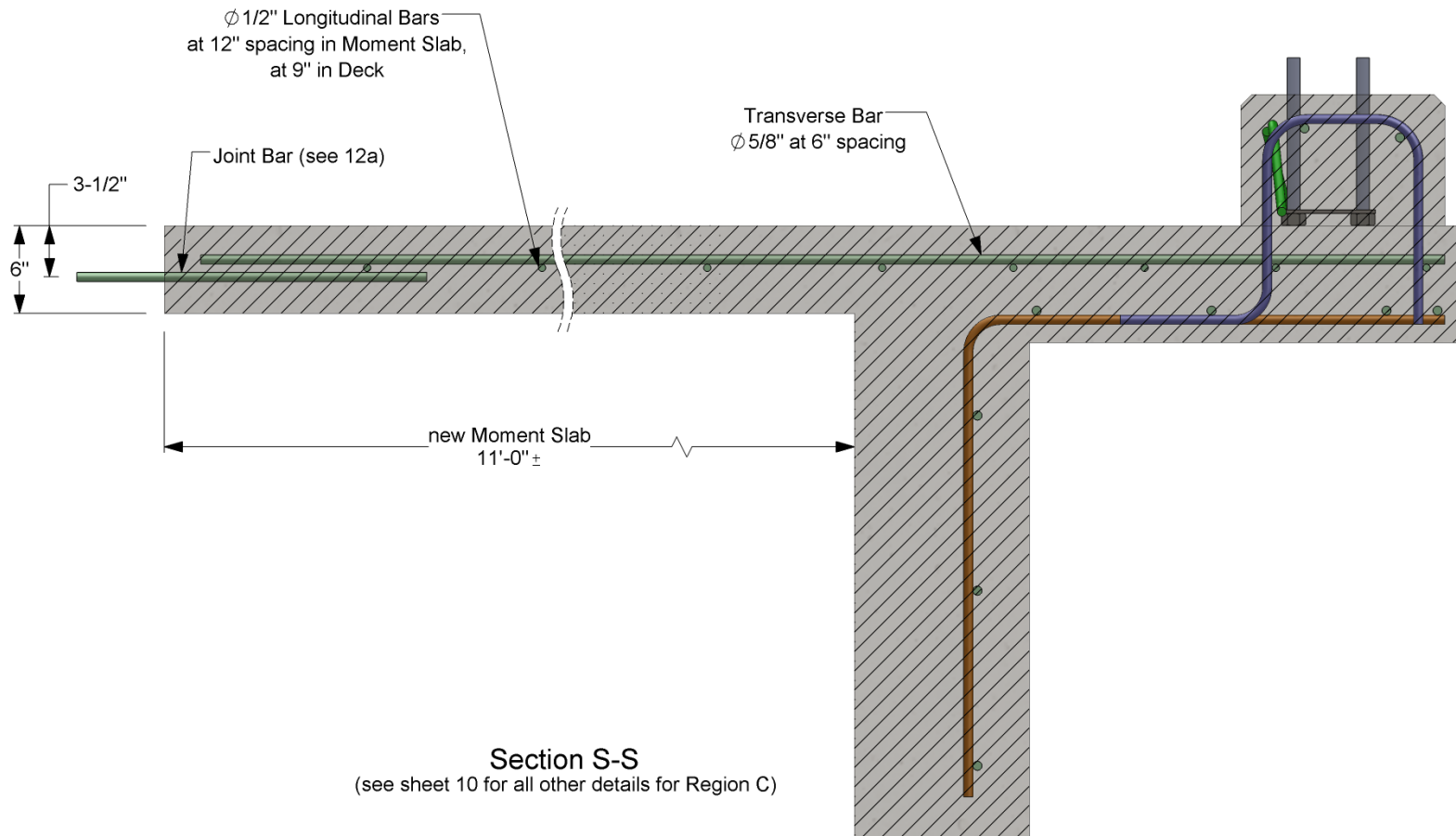
**11b.** Long Tie Bars spaced at 24" and welded to existing rebar protruding from the runway (not shown here).




Roadside Safety and  
Physical Security Division -  
Proving Ground

Project 490026-4-3	42" Picket Rail	2016-07-20
Drawn By GES	Scale: 1:20	Sheet 11 of 14 New Concrete

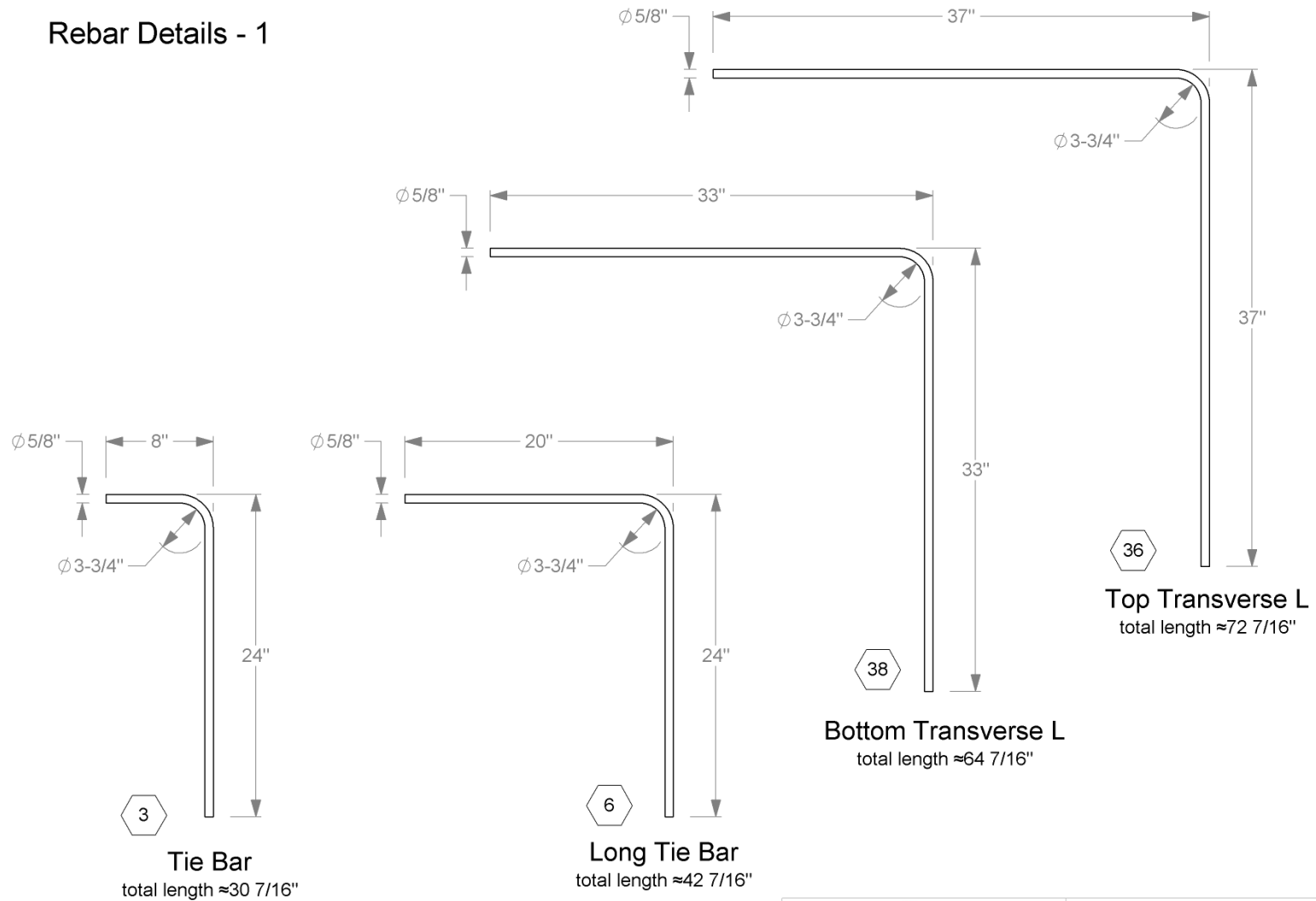




**12a.** Drill minimum 6" into existing concrete and secure Joint Bars with Hilti RE-500 epoxy, according to manufacturer's instructions. Space at 24".

		Roadside Safety and Physical Security Division - Proving Ground	
Project 490026-4-3	42" Picket Rail	2016-07-20	
Drawn By GES	Scale 1:10	Sheet 12 of 14	Section View

# Rebar Details - 1



13a. The numeral in the hexagon denotes the quantity needed for each Bar.

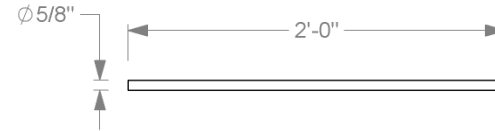
		Roadside Safety and Physical Security Division - Proving Ground	
Project 490026-4-3	42" Picket Rail	2016-07-20	
Drawn By GES	Scale 1:10	Sheet 13 of 14	Rebar Details - 1

T:\11-ProjectFiles\490026-TxDOT\4 - 42 inch Picket Rail - Williams\4-3\Drafting\490026-4-3 Drawing

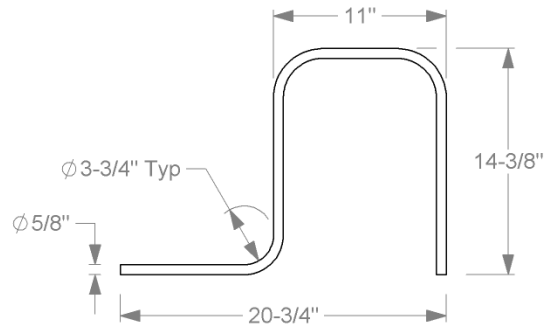
## Rebar Details - 2



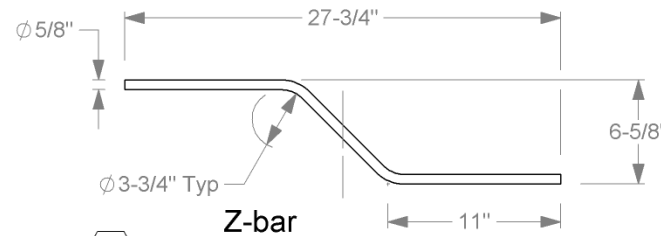
4 Rebar, #4 in new Wall



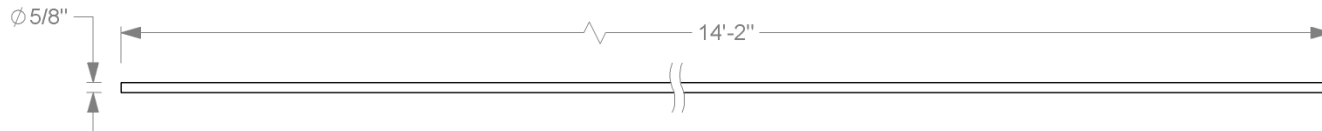
19 Joint Bar



112 Deck Stirrup  
total length  $\approx 44 \frac{7}{8}$ "



14 Z-bar  
total length  $\approx 30 \frac{1}{16}$ "



76 Moment Slab Transverse Bar

14a. The numeral in the hexagon denotes the quantity needed for each Bar.

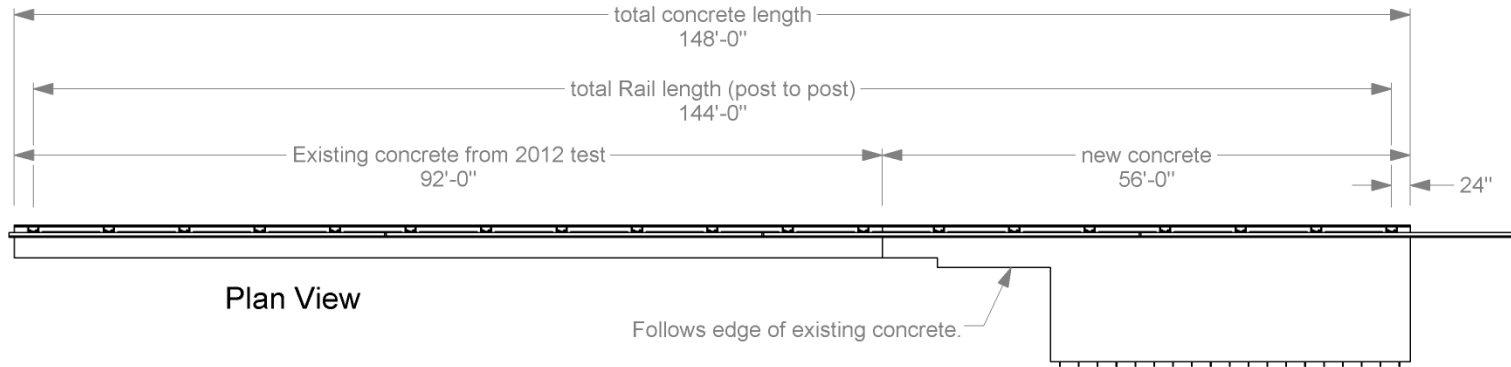


Roadside Safety and  
Physical Security Division -  
Proving Ground

Project 490026-4-3 42" Picket Rail 2016-07-20

Drawn By GES Scale 1:10 Sheet 14 of 14 Rebar Details - 2

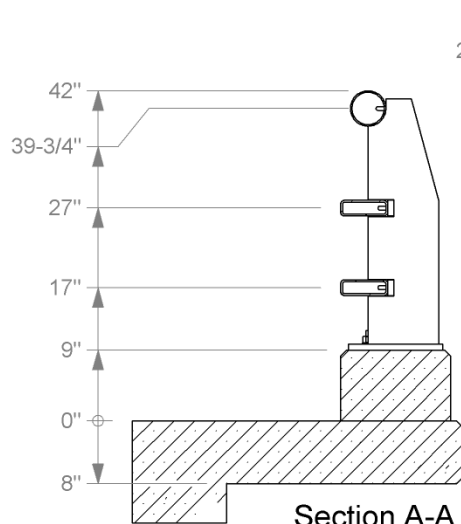
# Test Installation



Plan View

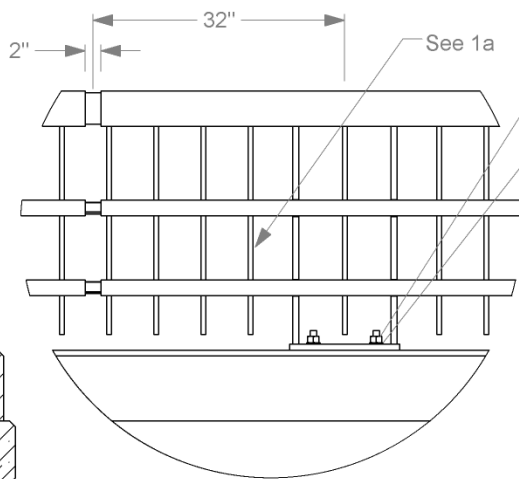


Elevation View



Section A-A

Scale 1 : 20  
(some dims rounded to nearest 1/8")



Detail B

Scale 1 : 20  
Typical each joint

- Nut, 7/8 heavy hex
- Typical x 4 at each Post
- Washer, 7/8 SAE Hardened

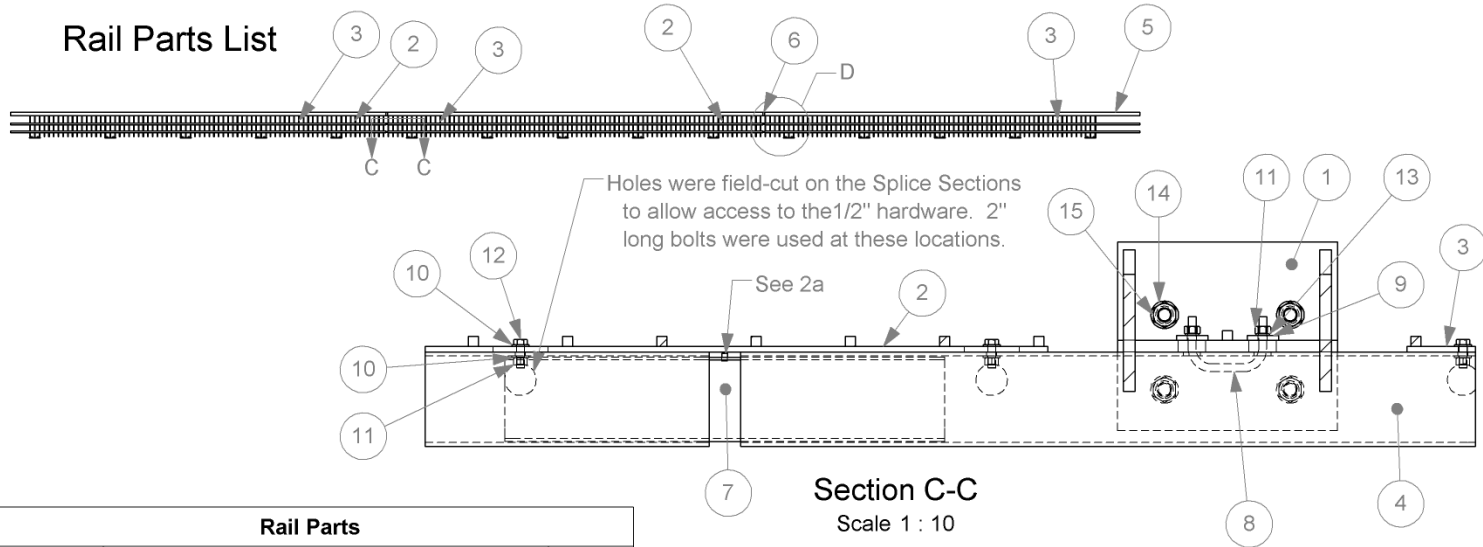
- 1a. Place Picket Panels with slots at Rail splice locations. Picket Panels with round holes go at all other locations.
- 1b. Tolerance on all fabricated parts is  $\pm 1/8$ " unless otherwise indicated.



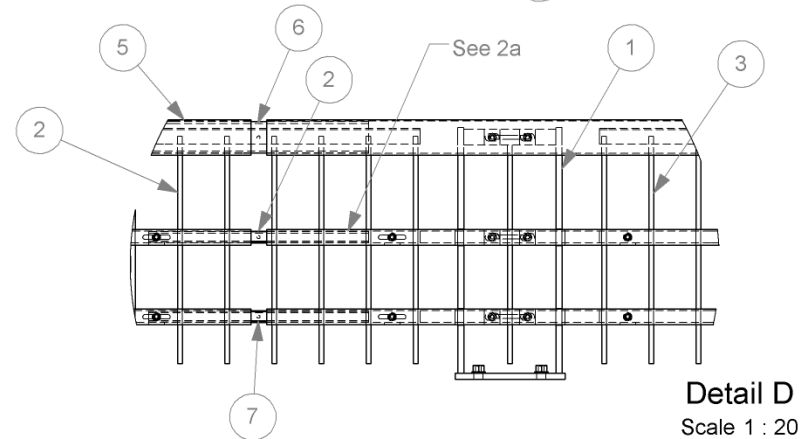
Roadside Safety and  
Physical Security Division -  
Proving Ground

Project 490026-4-3	42" Picket Rail	2016-07-20
Drawn By GES	Scale:1:200	Sheet 1 of 14 Test Installation

## Rail Parts List



Rail Parts		
#	Part Name	Qty.
1	Post for 42" Picket Rail	15
2	Picket Rail Panel, with slots	2
3	Picket Rail Panel, with holes	12
4	Rectangular Rail	6
5	Round Rail	3
6	Splice Section for Round Rail	2
7	Splice Section for Rectangular Rail	4
8	U-bolt for Picket Rail	45
9	Plate Washer for U-bolt	90
10	Washer, 1/2 SAE Hardened	168
11	Nut, 1/2 heavy hex	174
12	Bolt, 1/2" x 1 1/2" hex	84
13	Washer, 1/2 Lock	90
14	Washer, 7/8 SAE Hardened	60
15	Nut, 7/8 heavy hex	60



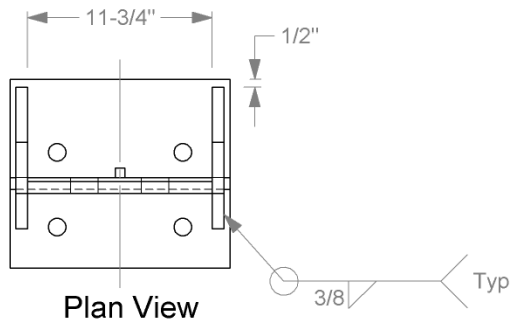
- 2a.** Place Splice Sections with Pins on Field Side.  
**2b.** Ø1/2" Bolts are ASTM A325. Ø7/8" Bolts are ASTM A449.



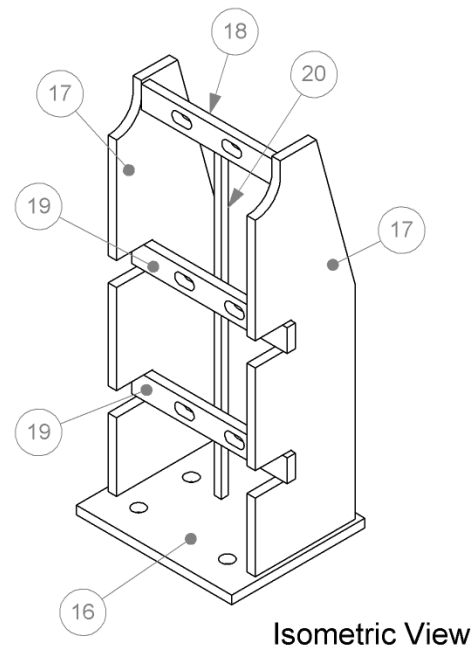
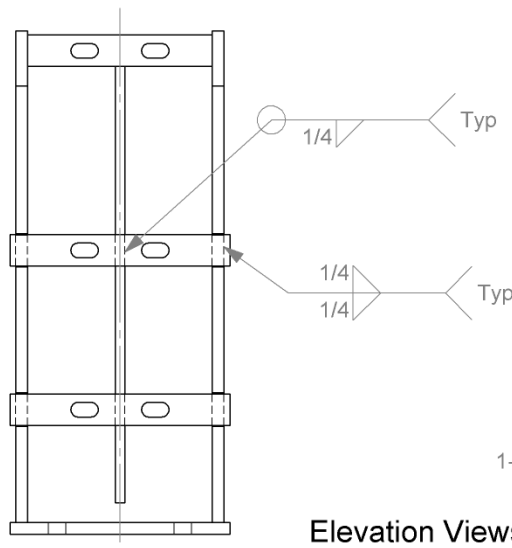
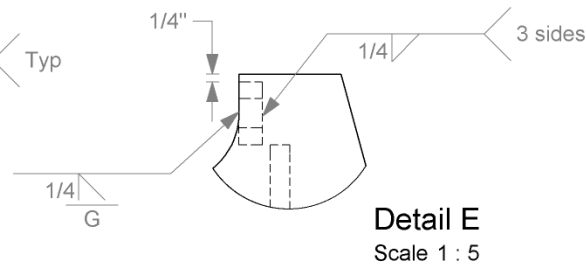
Roadside Safety and  
Physical Security Division -  
Proving Ground

Project 490026-4 / 1-2 42" Picket Rail 2016-07-18  
 Drawn By GES Scale:1:200 Sheet 2 of 14 Rail Parts List

# Post

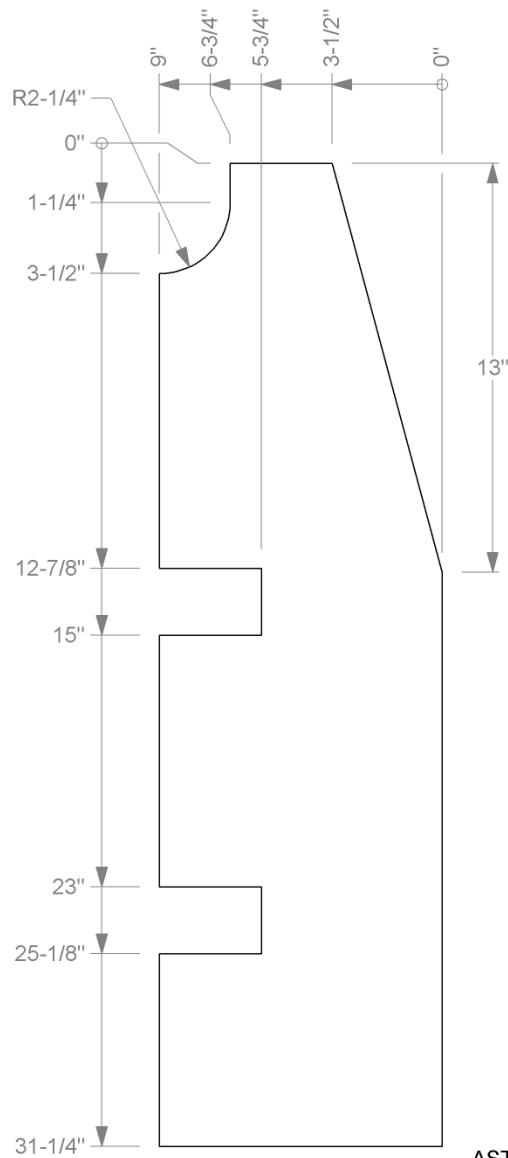


Post Parts					
#	Body Name	Description	Length	MATERIAL	Qty
16	Base Plate	Plate, 12" x 3/4"	14"	ASTM A572 Grade 50	1
17	Side Plate	Plate, 9" x 3/4"	31 1/4"	ASTM A572 Grade 50	2
18	Rail Plate, Top	Plate, 2" x 3/4"	11 3/4"	ASTM A36 Steel	1
19	Rail Plate, Bot and Mid	Plate, 2" x 3/4"	14"	ASTM A36 Steel	2
20	Picket	Plate, 5/8" x 5/8"	27 3/4"	ASTM A36 Steel	1



Roadside Safety and  
Physical Security Division -  
Proving Ground

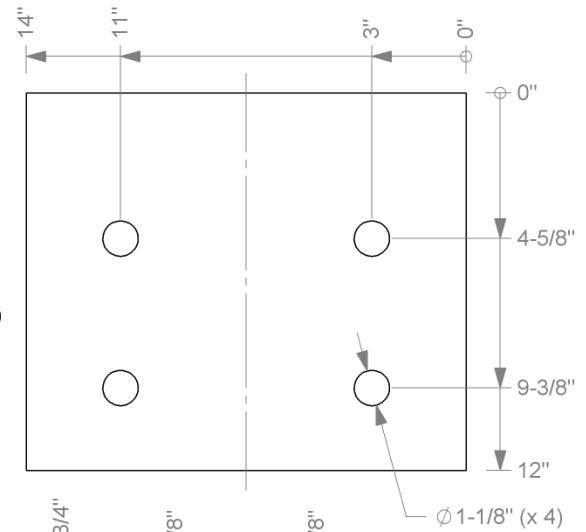
Project 490026-4 / 1-2 42" Picket Rail 2016-07-18  
 Drawn By GES Scale:1:500 Sheet 3 of 14 Post



### Post Parts

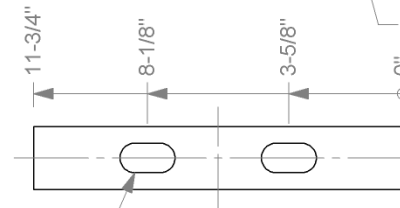
(See Table, previous sheet, for Picket details)

**Base Plate**  
Plate, 12" x 3/4"  
ASTM A572 Grade 50

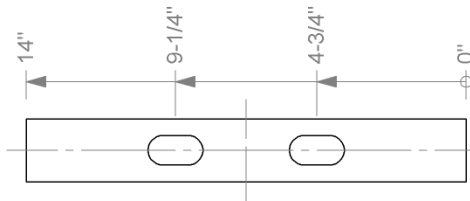


**Rail Plate, Top**  
Plate, 2" x 3/4"  
ASTM A36 Steel

Slots, 15/16" x 1-3/4"  
Typical both Rail Plates



**Rail Plate, Bot and Mid**  
Plate, 2" x 3/4"  
ASTM A36 Steel

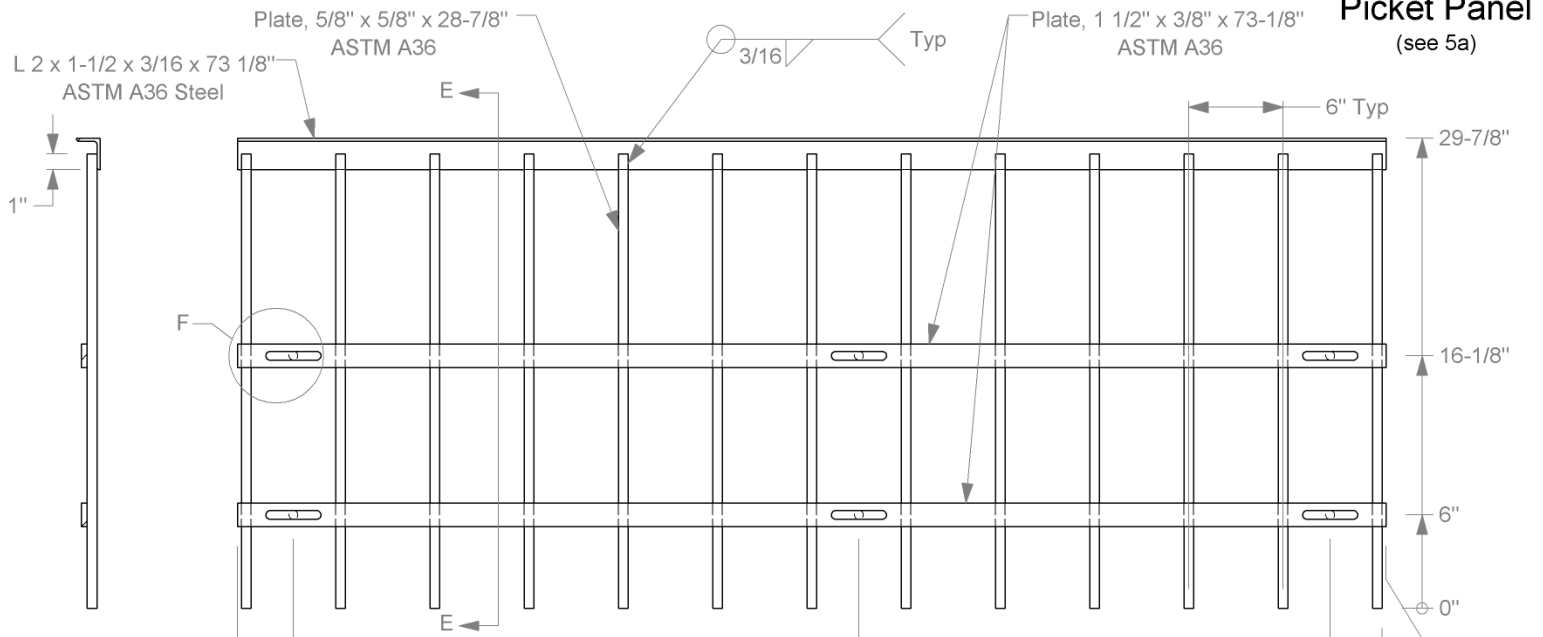


**Side Plate**  
Plate, 9" x 3/4"  
ASTM A572 Grade 50



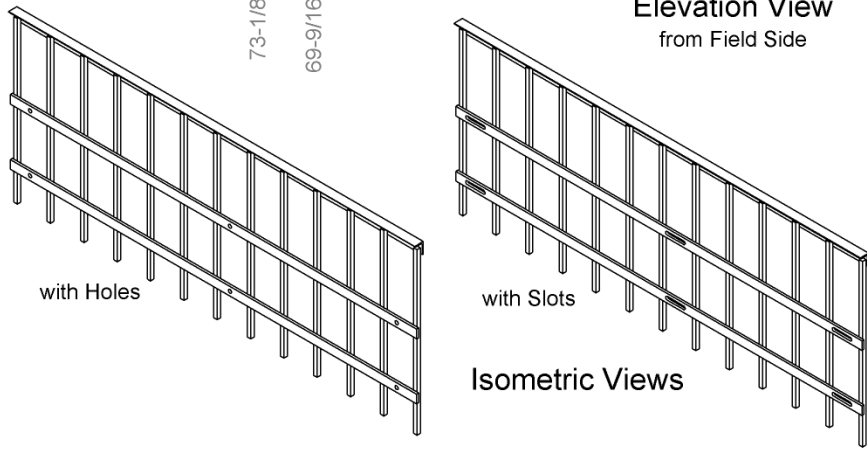
Roadside Safety and  
Physical Security Division -  
Proving Ground

Project 490026-4 / 1-2	42" Picket Rail	2016-07-18
Drawn By GES	Scale 1:5	Sheet 4 of 14 Post Parts

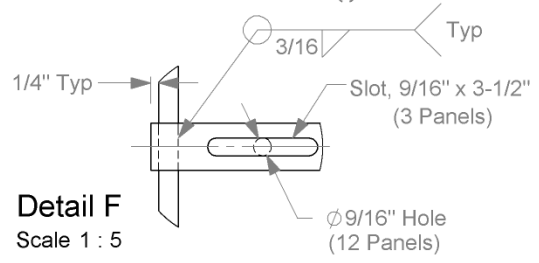


Section E-E

Elevation View  
from Field Side



Isometric Views



Detail F  
Scale 1:5

**Picket Panel**  
(see 5a)

5a. Need 18 Panels total, 3 with Slots and 15 with holes at same location.



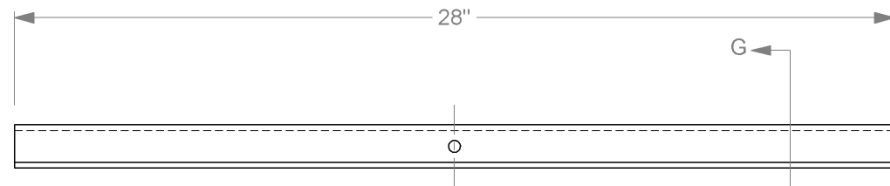
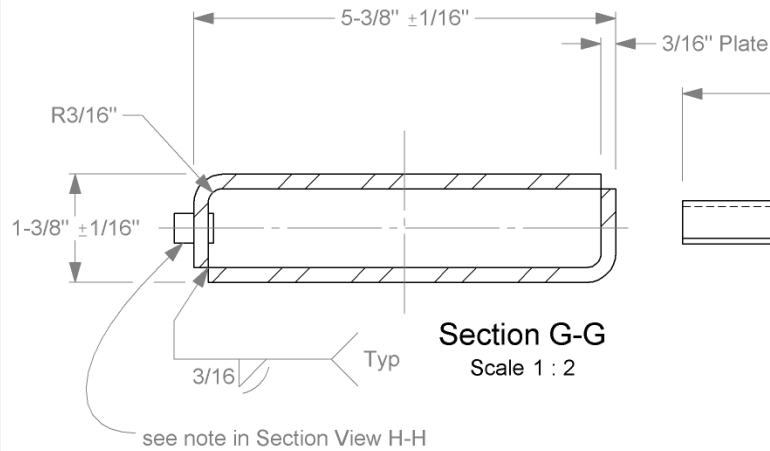
Roadside Safety and  
Physical Security Division -  
Proving Ground

Project 490026-4 / 1-2	42" Picket Rail	2016-07-18
Drawn By GES	Scale 1:10	Sheet 5 of 14 Picket Panel

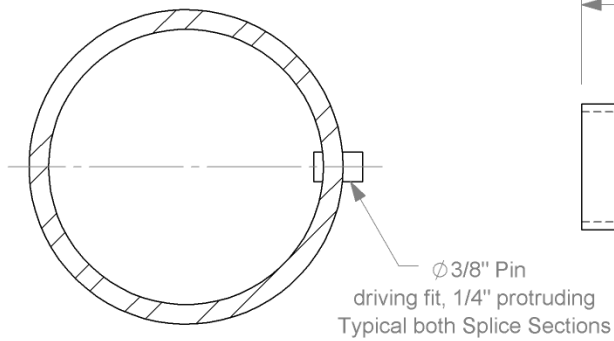
T:\11-ProjectFiles\490026-TxDOT\4 - 42 inch Picket Rail - Williams\4-2 Drafting\490026-4-2 Drawing



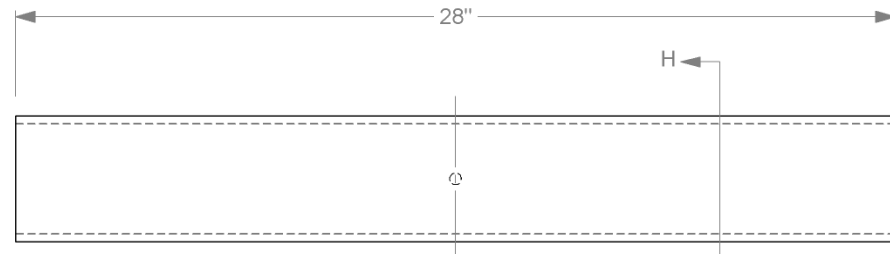
## Splice Sections



**Splice Section for Rectangular Rail**  
ASTM A36 Steel  
(see 6a)



**Section H-H**  
Scale 1 : 2



**Splice Section for Round Rail**  
HSS Round 4" x 1/4" x 28"  
ASTM A500 Grade B

**6a.** Check Splice Sleeve for Rectangular Rail for loose fit in Rectangular Rail after fabrication is completed.

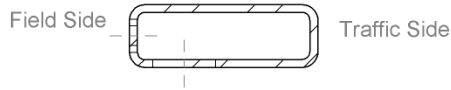
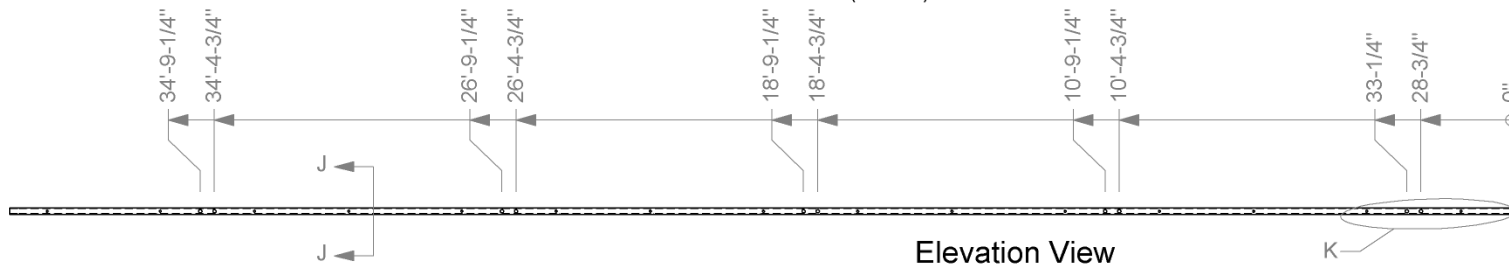
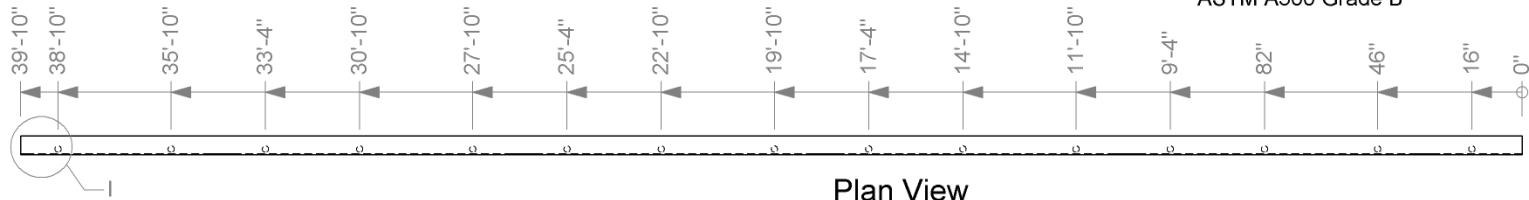


Roadside Safety and  
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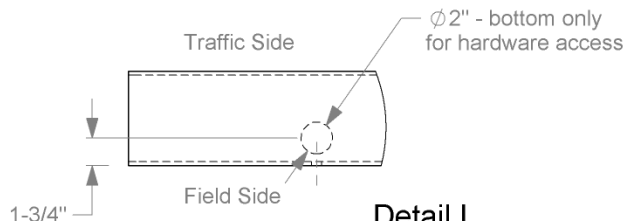
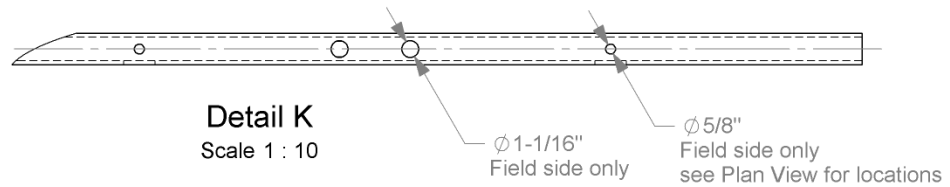
Project 490026-4 / 1-2 42" Picket Rail	2016-07-18
Drawn By GES	Scale 1:5 Sheet 6 of 14 Splice Sections

# Rectangular Rail

HSS 6" x 2" x 1/4"  
ASTM A500 Grade B

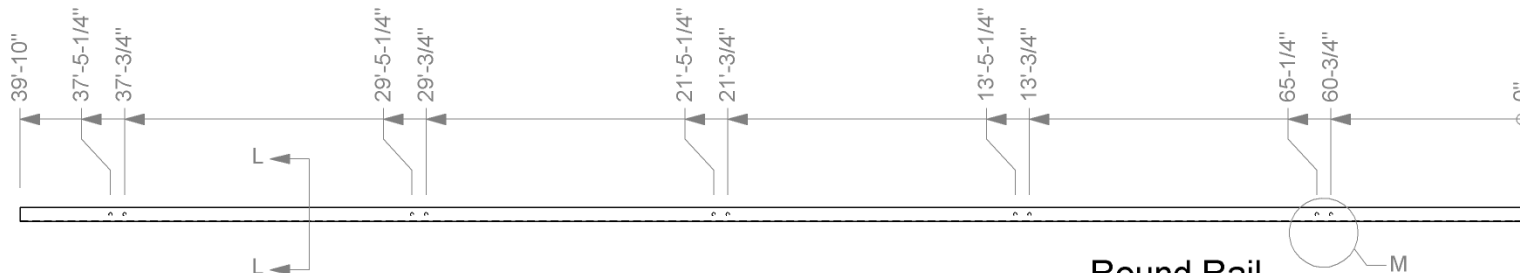


**Section J-J**  
Scale 1 : 5

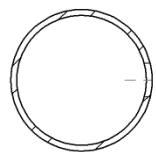


**7a.** Please note that the Plan View and Detail I are showing the Rail from the top. The holes are on bottom.

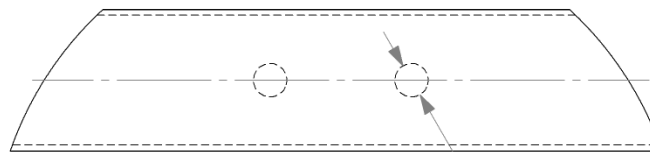
		Roadside Safety and Physical Security Division - Proving Ground	
Project 490026-4 / 1-2 42" Picket Rail		2016-07-18	
Drawn By GES	Scale 1:50	Sheet 7 of 14	Rectangular Rail



**Round Rail**  
 HSS Round 4 1/2" x 3/16"  
 ASTM A500 Grade B  
 Elevation View

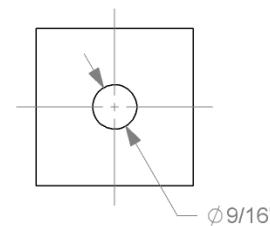


**Section L-L**  
 Scale 1 : 5

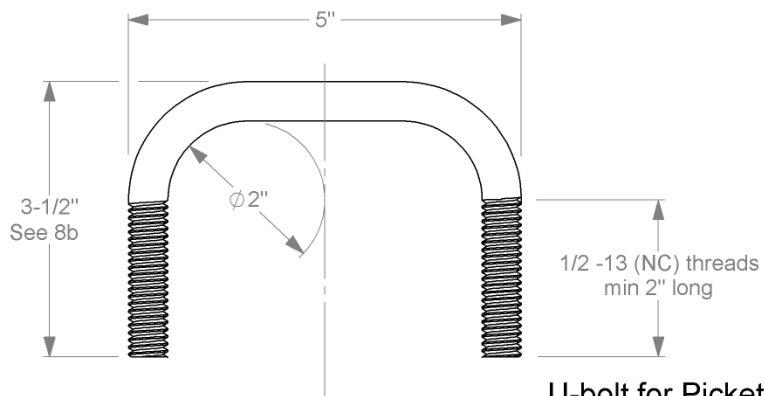


**Detail M**  
 Scale 1 : 5

Ø 1-1/16"  
 Field side only



**Plate Washer for U-bolt**  
 Plate, 2" x 5/16" x 2"  
 ASTM A36 Steel  
 Scale 1:2



**U-bolt for Picket Rail**  
 Ø 1/2" ASTM A36 Steel  
 Scale 1:2

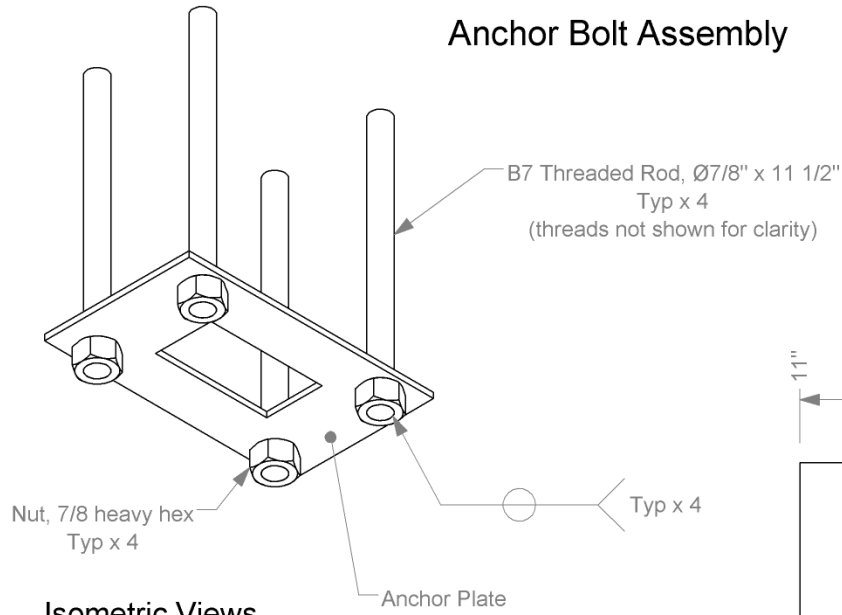
8a. Dimension is approximate. Rod is 10" long before bending.



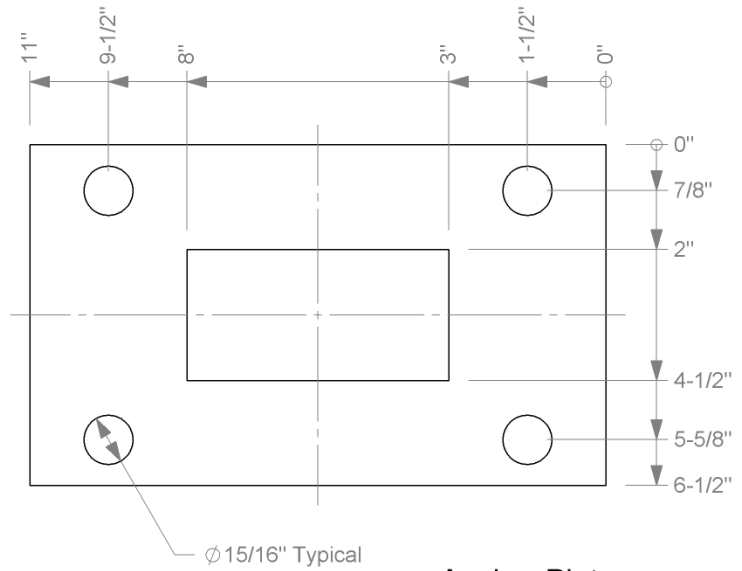
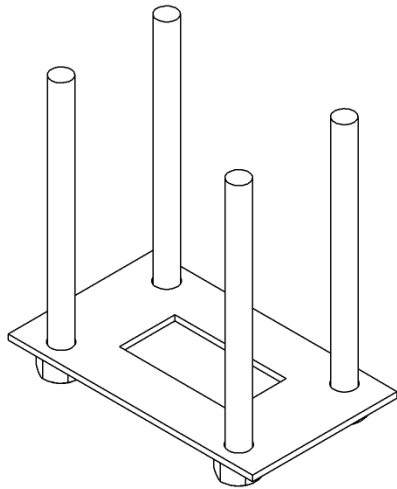
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Project 490026-4 / 1-2	42" Picket Rail	2016-07-18
Drawn By GES	Scale 1:50	Sheet 8 of 14 Round Rail, etc.

# Anchor Bolt Assembly



## Isometric Views



**Anchor Plate**  
Plate, 6 1/2" x 1/4" x 11"  
ASTM A36 Steel



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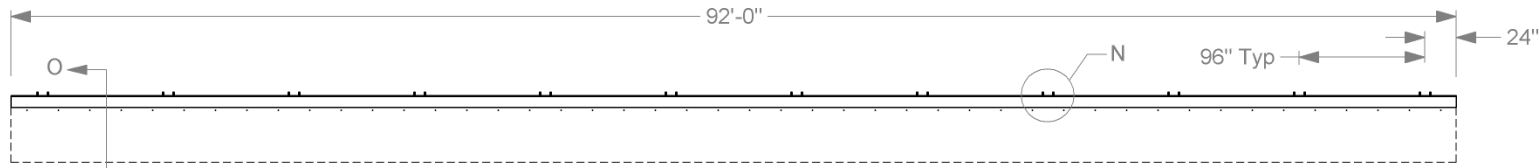
2016-07-18

Drawn By GES

Scale 1:3

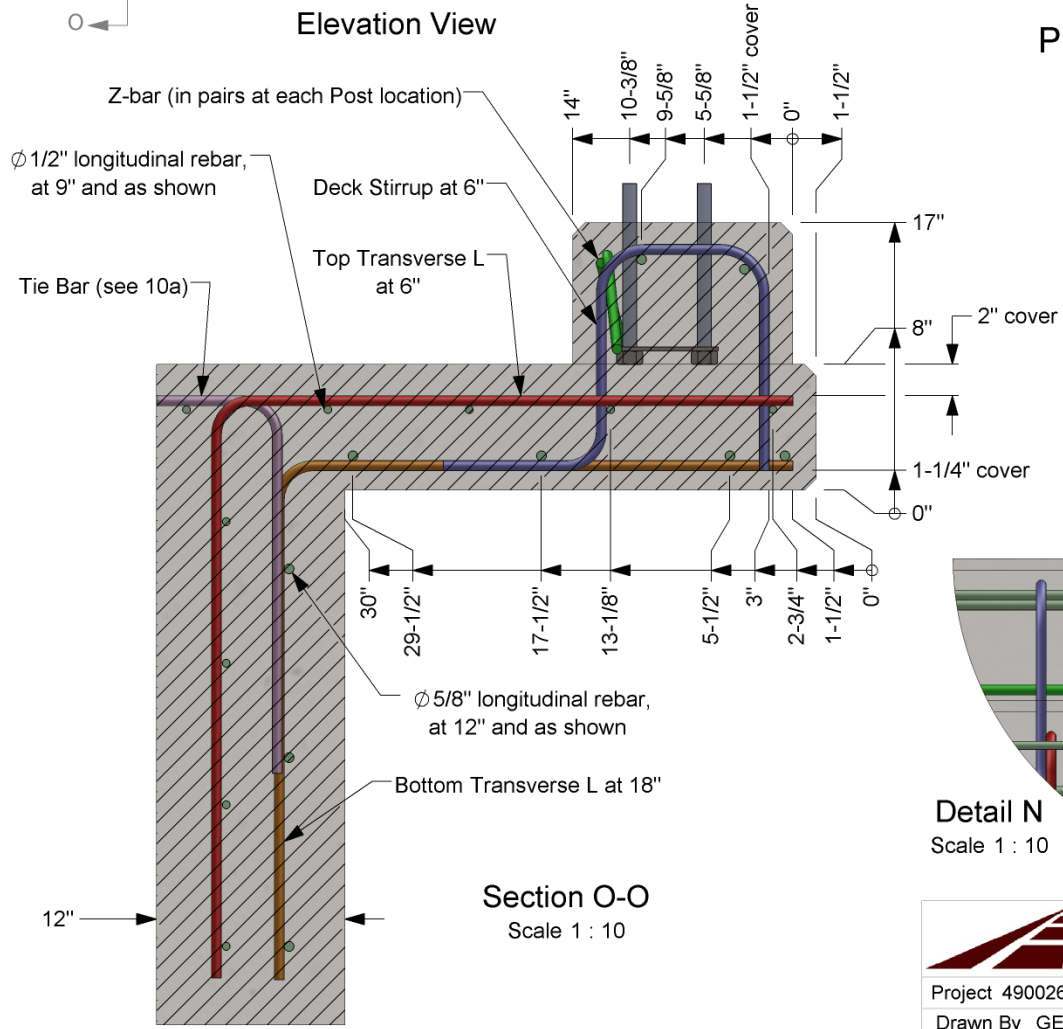
Sheet 9 of 14

Anchor Bolt Assembly



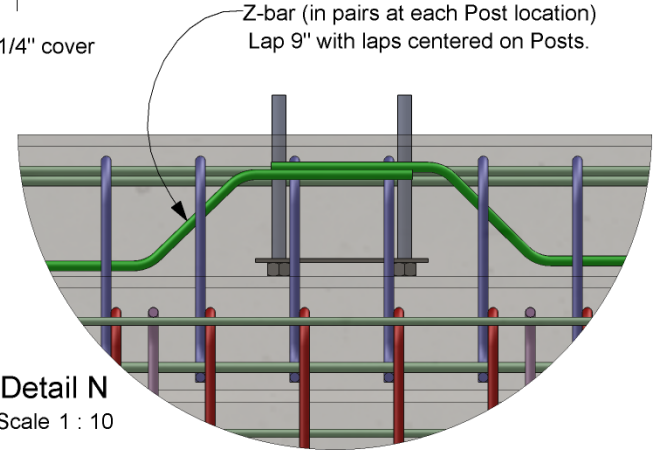
Elevation View

Previous Concrete



Section O-O  
Scale 1 : 10

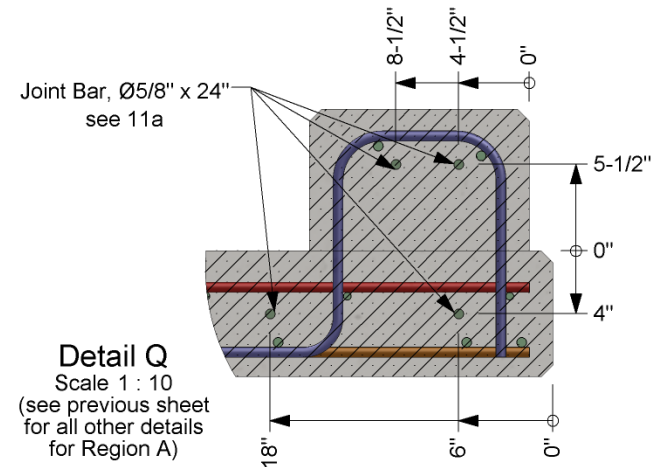
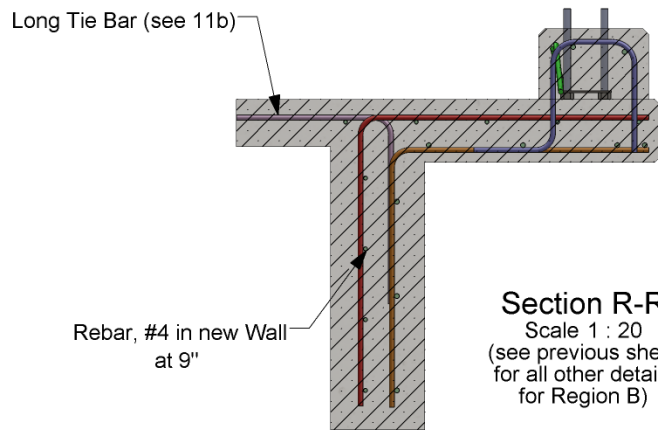
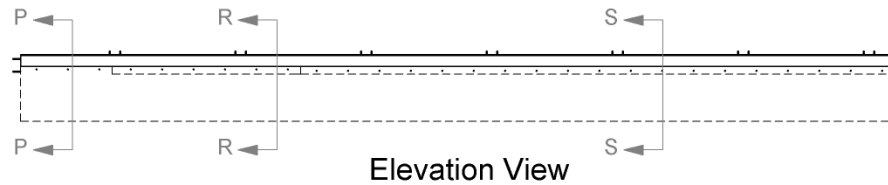
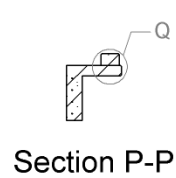
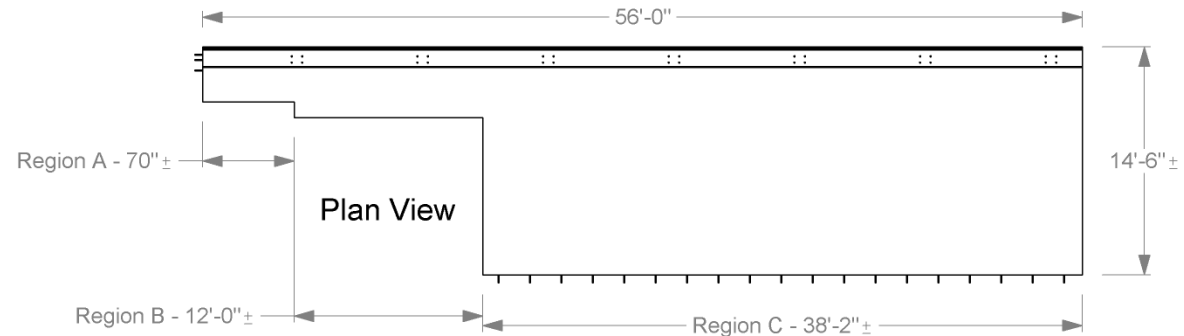
- 10a.** Tie Bars spaced at 24", and welded to existing rebar protruding from the runway (not shown here).
- 10b.** All Rebar is grade 60. Minimum lap distance is 17" for Ø1/2" bars and 21" for Ø5/8" bars.
- 10c.** Concrete is TxDOT Class S (4000 psi). Chamfer edges of Deck and Curb 3/4" as shown.



Detail N  
Scale 1 : 10

		Roadside Safety and Physical Security Division - Proving Ground	
Project	490026-4 / 1-2	42" Picket Rail	2016-07-18
Drawn By	GES	Scale: 1:120	Sheet 10 of 14 Previous Concrete

# New Concrete



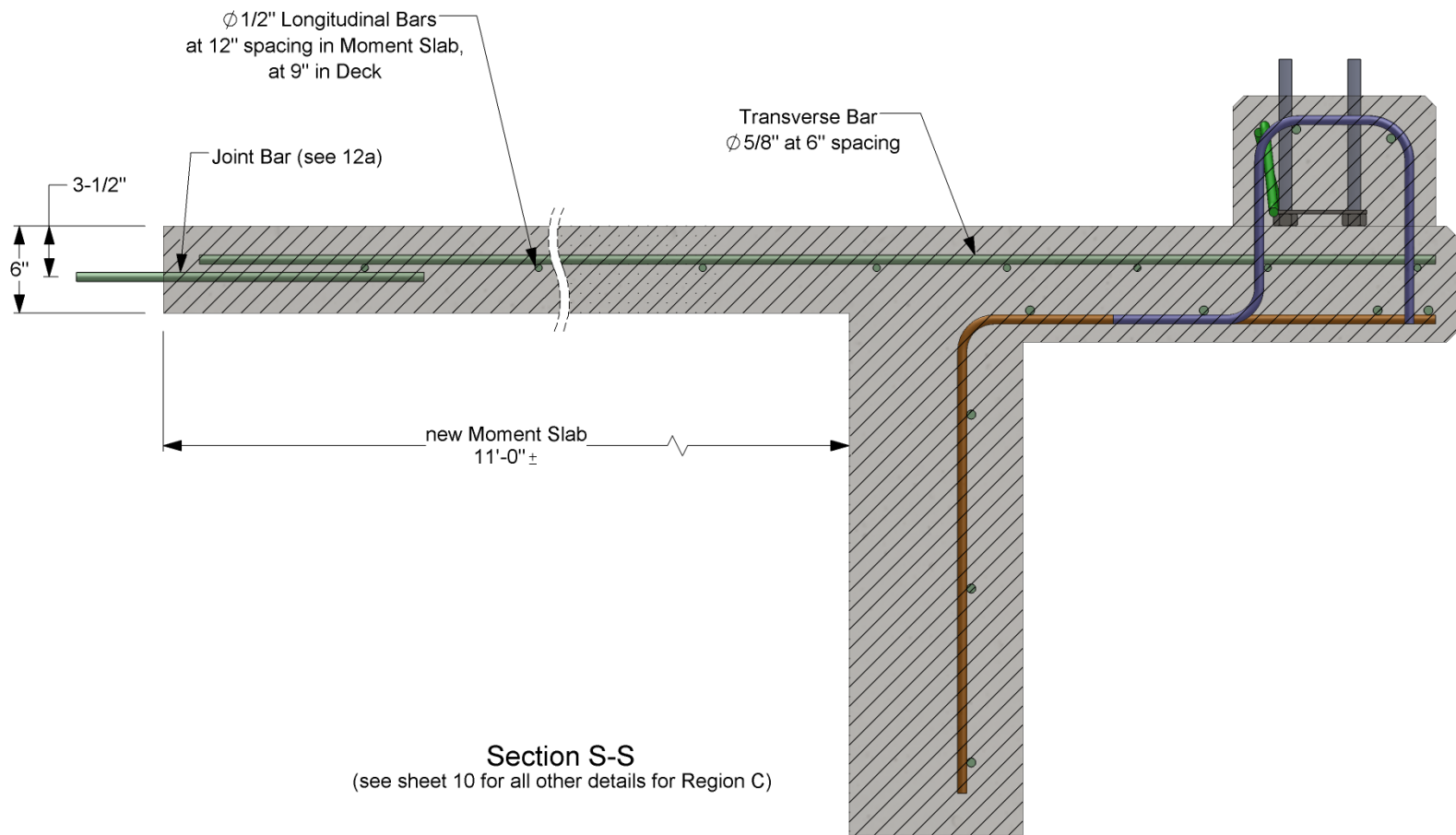
**11a.** Drill minimum 6" into existing concrete and secure Joint Bars with Hilti RE-500 epoxy, according to manufacturer's instructions.

**11b.** Long Tie Bars spaced at 24" and welded to existing rebar protruding from the runway (not shown here).



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Drawn By GES	Scale: 1:20	Sheet 11 of 14 New Concrete



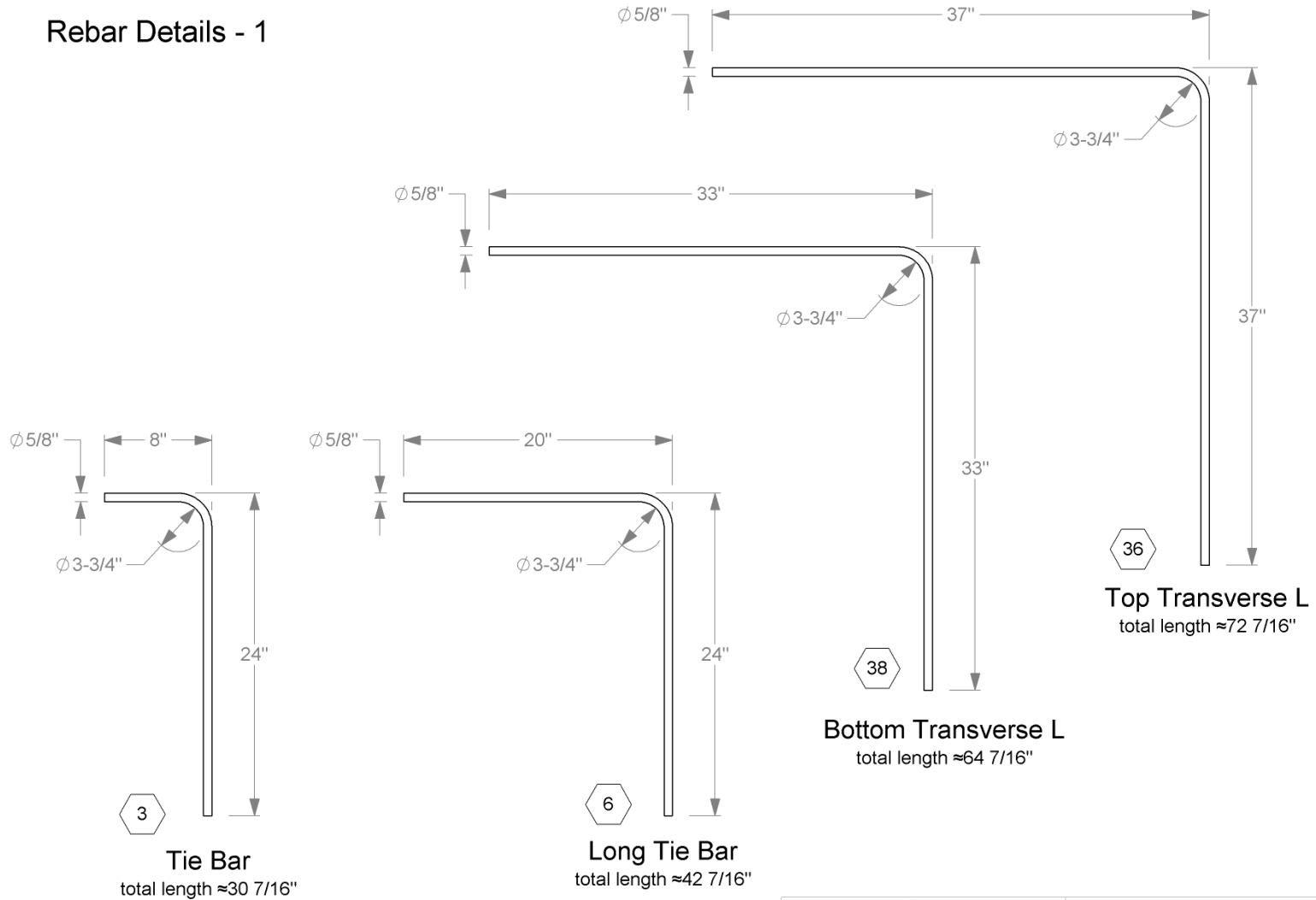
**12a.** Drill minimum 6" into existing concrete and secure Joint Bars with Hilti RE-500 epoxy, according to manufacturer's instructions. Space at 24".




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Drawn By GES	Scale 1:10	Sheet 12 of 14 Section View

# Rebar Details - 1



13a. The numeral in the hexagon denotes the quantity needed for each Bar.

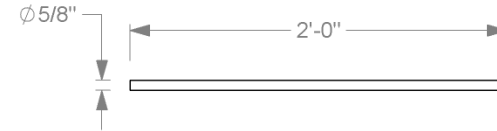
	Roadside Safety and Physical Security Division - Proving Ground	
	Project 490026-4 / 1-2 42" Picket Rail	2016-07-18
Drawn By GES	Scale 1:10	Sheet 13 of 14 Rebar Details - 1



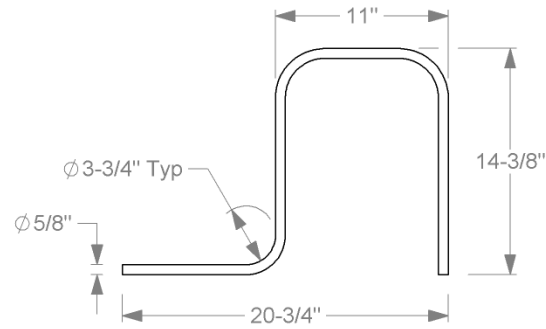
## Rebar Details - 2



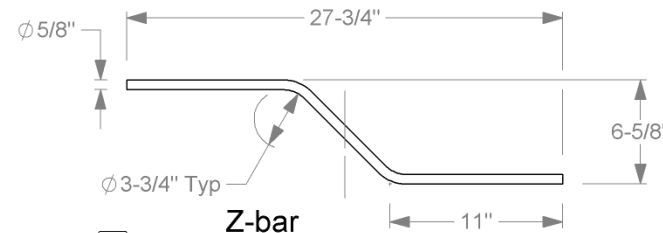
4 Rebar, #4 in new Wall



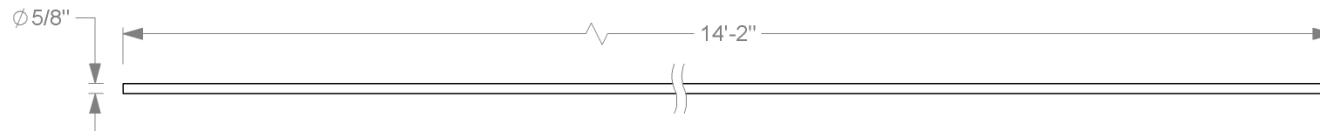
19 Joint Bar



112 Deck Stirrup  
total length  $\approx 44 \frac{7}{8}$ "



14 Z-bar  
total length  $\approx 30 \frac{1}{16}$ "



76 Moment Slab Transverse Bar

14a. The numeral in the hexagon denotes the quantity needed for each Bar.



Roadside Safety and  
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Project 490026-4 / 1-2 42" Picket Rail

2016-07-18

Drawn By GES

Scale 1:10

Sheet 14 of 14 Rebar Details - 2